

**HYDROLOGIC REPORT
EVANS CREEK BLOCK "N"
HYDROLOGIC INVESTIGATION**

Volume II - Technical Appendix

Washoe County, NV
For
Washoe County Water Resources Department



Nimbus Job No. 9912
July, 1999



Nimbus Engineers

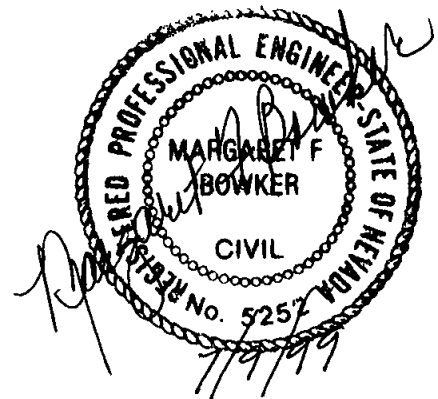
3785 Baker Ln., Suite 201 • Reno, NV 89509
Mail: P.O. Box 10220 • Reno, NV 89510
(775) 689-8630 • Fax (775) 689-8614
Email: nimbus@intercomm.com

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APPENDICES

Appendix A. Rainfall Depth Calculations

Appendix B. SCS Runoff Curve Number Calculations

Appendix C. Lag Time/Time of Concentration Calculations

Appendix D. HEC-1 Model Results and Hydrographs

Appendix A

✓

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT

Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

Sub-Basin	Sub-area, (miles ²)			Sub-area Precip Depth (in)			D(2,1) Precip*Area			D(2,6) Precip*Area		
	1 hour	6 hour	24hour	1 hour	6 hour	24hour	1 hour	6 hour	24hour	1 hour	6 hour	24hour
E1	A1	0.77	0.77	0.29	0.40	0.90	1.40	0.31	0.69	0.41		
	A2	0.00	0.00	0.48	0.40	0.80	1.30	0.00	0.00	0.62		
Total Sub-basin Area		0.77	0.77	0.77								
Weighted Precip.								0.40	0.90	1.34		
E2	A1	1.20	0.21	0.405	0.40	0.90	1.30	0.48	0.19	0.53		
	A2	0.00	0.99	0.792	0.40	0.80	1.20	0.00	0.79	0.95		
Total Sub-basin Area		1.20	1.20	1.20								
Weighted Precip.								0.40	0.82	1.23		
E3	A1	1.51	0.37	0.953	0.40	0.90	1.30	0.60	0.33	1.24		
	A2	0.00	1.14	0.557	0.40	0.80	1.20	0.00	0.91	0.67		
Total Sub-basin Area		1.51	1.51	1.51								
Weighted Precip.								0.40	0.82	1.26		
E4	A1	0.58	0.58	0.45	0.40	0.80	1.20	0.23	0.46	0.54		
	A2	0.00	0.00	0.13	0.40	0.70	1.10	0.00	0.00	0.14		
Total Sub-basin Area		0.58	0.58	0.58								
Weighted Precip.								0.40	0.79	1.17		
E5	A1	0.31	0.31	0.026	0.40	0.80	1.20	0.12	0.25	0.03		
	A2	0.00	0.00	0.284	0.40	0.70	1.10	0.00	0.00	0.31		
Total Sub-basin Area		0.31	0.31	0.31								
Weighted Precip.								0.39	0.81	1.10		

Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT

Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

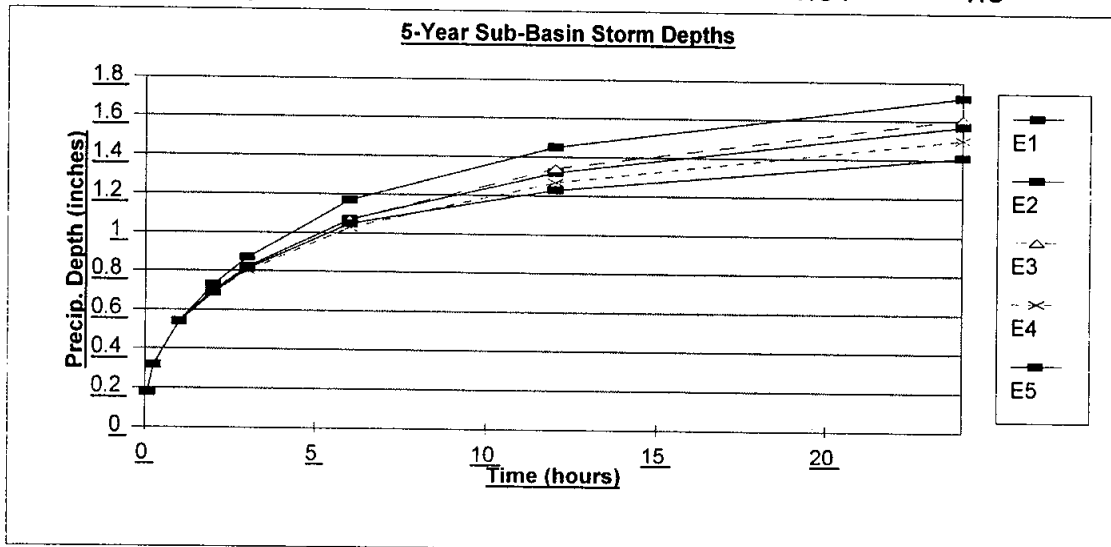
Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.40	0.40	0.40	0.40	0.40
D(2,6)	0.90	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.10

INPUT

	RGF		RATIO
(5,1 hour)	1.36	(5, 5 min)	0.33
(5,6 hour)	1.30	(5, 15 min)	0.60
(5,24 hour)	1.28		

OUTPUT

	Time (hr)	Sub-Basin 5-Year Precip Depth				
		E1	E2	E3	E4	E5
(5,5 min)	0.08	0.18	0.18	0.18	0.18	0.18
(5,15 min)	0.25	0.32	0.32	0.32	0.32	0.32
(5,1 hour)	1	0.54	0.54	0.54	0.54	0.54
(5,2 hour)	2	0.73	0.70	0.70	0.69	0.69
(5,3 hour)	3	0.87	0.82	0.82	0.80	0.81
(5,6 hour)	6	1.17	1.07	1.07	1.03	1.05
(5,12 hour)	12	1.45	1.32	1.34	1.27	1.23
(5,24 hour)	24	1.72	1.57	1.61	1.5	1.41



Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT
Methodology per the Washoe County Criteria and Drainage Manual

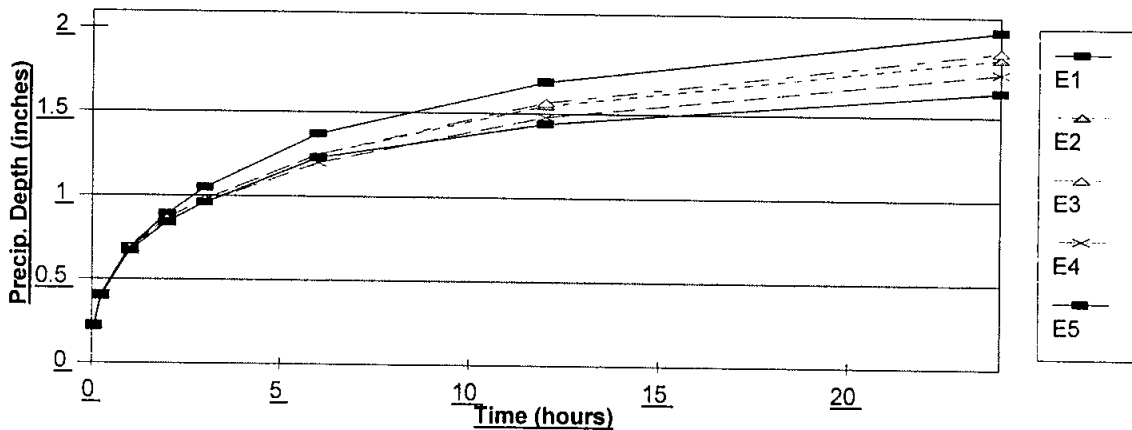
1) Calculate average D(2,X) storm depth using isohyetal method

Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.40	0.40	0.40	0.40	0.39
D(2,6)	0.90	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.10

INPUT	RGF	RATIO
(10,1 hour)	1.72	(10, 5 min) 0.33
(10,6 hour)	1.52	(10, 15 min) 0.60
(10,24 hour)	1.50	

OUTPUT	Time (hr)	Sub-Basin 10-Year Precip Depth				
		E1	E2	E3	E4	E5
(10,5 min)	0.08	0.23	0.23	0.23	0.23	0.22
(10,15 min)	0.25	0.41	0.41	0.41	0.41	0.4
(10,1 hour)	1	0.69	0.69	0.69	0.69	0.67
(10,2 hour)	2	0.89	0.86	0.86	0.84	0.84
(10,3 hour)	3	1.05	0.98	0.98	0.96	0.96
(10,6 hour)	6	1.37	1.25	1.25	1.2	1.23
(10,12 hour)	12	1.69	1.55	1.57	1.48	1.44
(10,24 hour)	24	2.01	1.85	1.89	1.76	1.65

10-Year Sub-Basin Storm Depths



Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT
Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

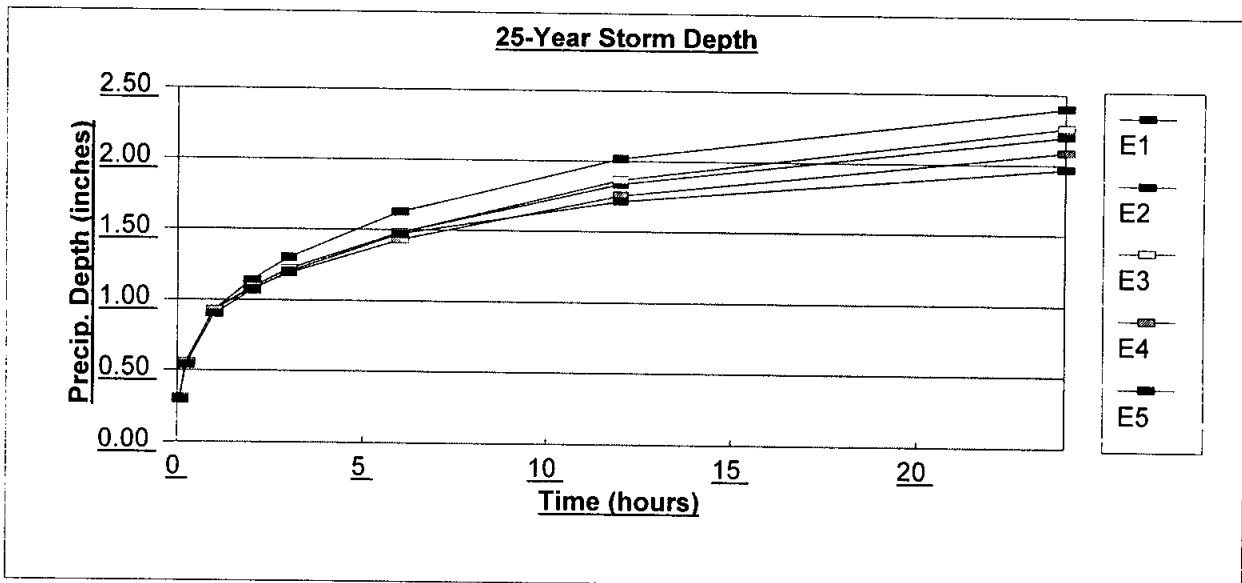
Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.4	0.4	0.4	0.4	0.39
D(2,6)	0.9	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.1

INPUT

	RGF		RATIO
(25,1 hour)	2.32	(25, 5 min)	0.33
(25,6 hour)	1.81	(25, 15 min)	0.60
(25,24 hour)	1.79		

OUTPUT

	Time (hrs)	E1	E2	E3	E4	E5
(25,5 min)	0.08	0.31	0.31	0.31	0.31	0.30
(25,15 min)	0.25	0.56	0.56	0.56	0.56	0.54
(25,1 hour)	1	0.93	0.93	0.93	0.93	0.90
(25,2 hour)	2	1.14	1.09	1.09	1.08	1.07
(25,3 hour)	3	1.30	1.22	1.22	1.19	1.20
(25,6 hour)	6	1.63	1.48	1.48	1.43	1.47
(25,12 hour)	12	2.02	1.84	1.87	1.76	1.72
(25,24 hour)	24	2.40	2.20	2.26	2.09	1.97



Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT

Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

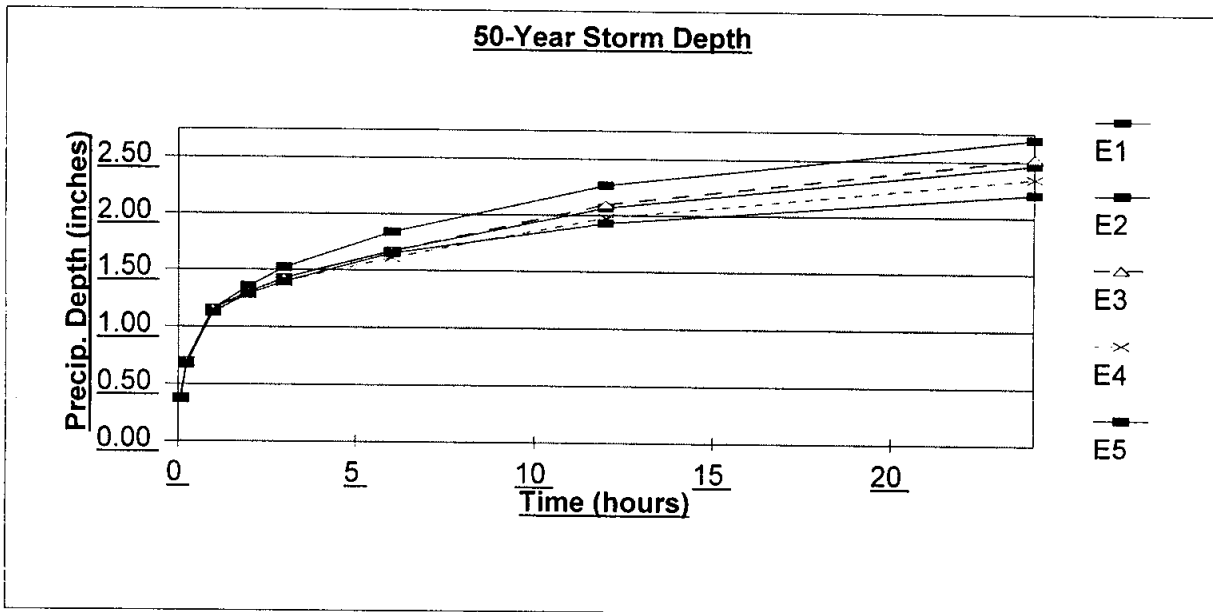
Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.4	0.4	0.4	0.4	0.39
D(2,6)	0.9	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.1

INPUT

	RGF		RATIO
(50,1 hour)	2.91	(50, 5 min)	0.33
(50,6 hour)	2.04	(50, 15 min)	0.60
(50,24 hour)	2.01		

OUTPUT

	Time (hrs)	E1	E2	E3	E4	E5
(50,5 min)	0.08	0.38	0.38	0.38	0.38	0.37
(50,15 min)	0.25	0.70	0.70	0.70	0.70	0.68
(50,1 hour)	1	1.16	1.16	1.16	1.16	1.13
(50,2 hour)	2	1.36	1.31	1.31	1.29	1.29
(50,3 hour)	3	1.52	1.43	1.43	1.40	1.40
(50,6 hour)	6	1.84	1.67	1.67	1.61	1.65
(50,12 hour)	12	2.27	2.07	2.10	1.98	1.93
(50,24 hour)	24	2.69	2.47	2.53	2.35	2.21



Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT
Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

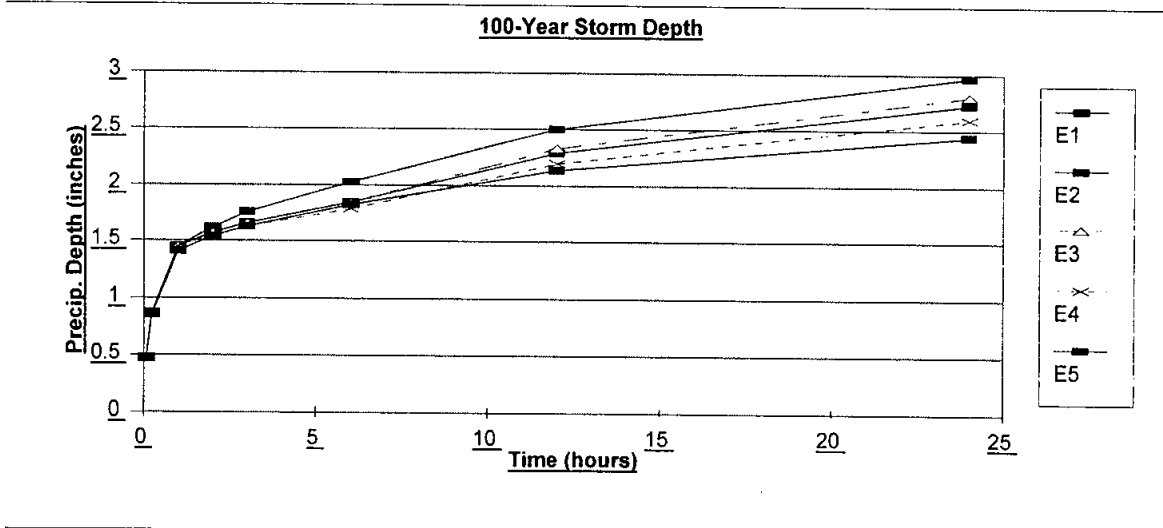
Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.4	0.4	0.4	0.4	0.39
D(2,6)	0.9	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.1

INPUT

	RGF		RATIO
(100,1 hour)	3.62	(100, 5 min)	0.33
(100,6 hour)	2.26	(100, 15 min)	0.60
(100,24 hour)	2.22		

OUTPUT

	Time (hrs)	E1	E2	E3	E4	E5
(100,5 min)	0.08	0.48	0.48	0.48	0.48	0.47
(100,15 min)	0.25	0.87	0.87	0.87	0.87	0.85
(100,1 hour)	1	1.45	1.45	1.45	1.45	1.41
(100,2 hour)	2	1.62	1.57	1.57	1.55	1.54
(100,3 hour)	3	1.76	1.66	1.66	1.63	1.63
(100,6 hour)	6	2.03	1.85	1.85	1.79	1.83
(100,12 hour)	12	2.50	2.29	2.33	2.2	2.14
(100,24 hour)	24	2.97	2.73	2.8	2.6	2.44



Evans Creek Block N Hydrology

RAINFALL DISTRIBUTION CALCULATION FOR HEC-1 INPUT
Methodology per the Washoe County Criteria and Drainage Manual

1) Calculate average D(2,X) storm depth using isohyetal method

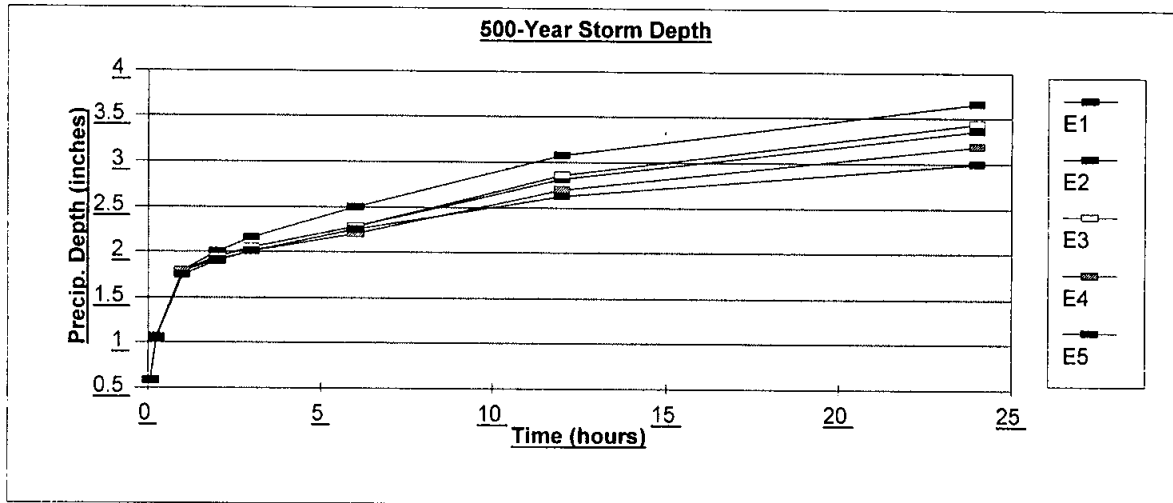
Sub-Basin	E1	E2	E3	E4	E5
D(2,1)	0.4	0.4	0.4	0.4	0.39
D(2,6)	0.9	0.82	0.82	0.79	0.81
D(2,24)	1.34	1.23	1.26	1.17	1.1

INPUT

	RGF		RATIO
(500,1 hour)	4.48	(500, 5 min)	0.33
(500,6 hour)	2.78	(500, 15 min)	0.60
(500,24 hour)	2.73		

OUTPUT

	Time (hrs)	E1	E2	E3	E4	E5
(500,5 min)	0.08	0.59	0.59	0.59	0.59	0.58
(500,15 min)	0.25	1.07	1.07	1.07	1.07	1.05
(500,1 hour)	1	1.79	1.79	1.79	1.79	1.75
(500,2 hour)	2	2.00	1.94	1.94	1.91	1.9
(500,3 hour)	3	2.16	2.05	2.05	2.01	2.01
(500,6 hour)	6	2.5	2.28	2.28	2.2	2.25
(500,12 hour)	12	3.08	2.82	2.86	2.7	2.63
(500,24 hour)	24	3.66	3.36	3.44	3.19	3



Appendix B

1

Worksheet 2: Runoff curve number

Project: Evans Creek Block N Hydrology By: CA Date: 06/11

Location: Phase II Sub-Basin E1 Chk: Date:

Mark 1 Undeveloped Developed

1 Runoff curve number (CN) **Note: Sage/Grass CN's per Draft Sparks Manual; Fig. 702**

Soil Name and hydrologic group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN 1 per line		Area Mark 1 acres sq. mi. percent	Product of CN X area
		Table Fig. 702	Fig. 2-4		
A	N/A		N/A	0%	N/A
B	Sagebrush/grass with 40% vegetative cover density		56	7%	3.92
C	Sagebrush/grass with 40% vegetative cover density		68	14%	9.52
D	Sagebrush/grass with 40% vegetative cover density		76	79%	60.04
Totals =				100%	73.48
CN (weighted) = (total product) / (total area) =		73.48		Use CN =	73
Total Sub-basin Area =				0.772	sq. miles

Developed Land Use	Soil Name and hydrologic group	CN per Table 702	Area sq. miles	% of Total Area	Product of CN X area
RES 1/3 acre	D	86	0.078	10.10%	8.7
RES 1/3 acre	D	86	0.051	6.61%	5.7
COM	D	93	0.029	3.76%	3.5
Undeveloped	D	73	0.614	79.53%	58.1
				100.00%	
Total Developed Land Use Weighted CN =					76.0

Worksheet 2: Runoff curve number

Project: Evans Creek Block N Hydrology By: CA Date: 06/11

Location: Phase II Sub-Basin E2 Chk: Date:

Mark 1 Undeveloped Developed

1 Runoff curve number (CN) **Note: Sage/Grass CN's per Draft Sparks Manual; Fig. 702**

Soil Name and hydrologic group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN			Area Mark 1 acres sq. mi. percent	Product of CN X area
		1 per line Table 702	Fig. 702	Fig. 2-4		
A	N/A			N/A	N/A	N/A
B	Sagebrush/grass with 40% vegetative cover density		56		17%	9.52
C	Sagebrush/grass with 40% vegetative cover density		68		6%	4.08
D	Sagebrush/grass with 40% vegetative cover density		76		77%	58.52
				Totals =	100%	72.12
CN (weighted) = (total product) / (total area) =		72.12		Use CN =	72	
				Total Sub-basin Area =	1.200 sq. miles	

Developed Land Use	Soil Name and hydrologic group	CN per Table 702	Area sq. miles	% of Total Area	Product of CN X area
RES 1/3 acre	D	86	0.256	21.33%	18.3
RES 1 acre	D	84	0.039	3.25%	2.7
IND	D	92	0.037	3.08%	2.8
COM	D	93	0.027	2.25%	2.1
COM	D	93	0.029	2.42%	2.2
Undeveloped	D	72	0.812	67.67%	48.7
				100.00%	
Total Developed Land Use Weighted CN =					77.0

Worksheet 2: Runoff curve number

Project: Evans Creek Block N Hydrology		By: CA		Date: 06/11		
Location: Phase II Sub-Basin E3		Chk:		Date:		
Mark 1	Undeveloped	X	Developed			
1 Runoff curve number (CN)		Note: Sage/Grass CN's per Draft Sparks Manual; Fig. 702				
Soil Name and hydrologic group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connectiod impervious area ratio)	CN			Area Mark 1 acres sq. mi. percent	Product of CN X area
		1 per line Table Fig. 702	Fig. 702	Fig. 2-4		
A	N/A			N/A	0%	N/A
B	Sagebrush/grass with 25% vegetative cover density			63.5	7%	4.45
C	Sagebrush/grass with 25% vegetative cover density			76	10%	7.6
D	Sagebrush/grass with 25% vegetative cover density			82	83%	68.06
Totals =					100%	80.11
CN (weighted) = (total product) / (total area) =		80.105		Use CN =	80	
Total Sub-basin Area =					1.510	sq. miles
Developed Land Use	Soil Name and hydrologic group	CN per Table 702	Area sq. miles	% of Total Area	Product of CN X area	
RES 1 acre	D	84	0.078	5.17%	4.3	
RES 1/8 acre	D	92	0.051	3.38%	3.1	
Undeveloped	D	80	1.381	91.46%	73.2	
				100.00%		
Total Developed Land Use Weighted CN =					81.0	

Worksheet 2: Runoff curve number

Project: Evans Creek Block N Hydrology		By: CA		Date: 06/11	
Location: Phase II Sub-Basin E4		Chk:		Date:	
Mark 1	Undeveloped	X	Developed		
1 Runoff curve number (CN)		Note: Sage/Grass CN's per Draft Sparks Manual; Fig. 702			
Soil Name and hydrologic group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN 1 per line Table 702 Fig. 702 Fig. 2-4		Area Mark 1 acres sq. mi. percent	Product of CN X area
A	N/A	N/A		0%	N/A
B	Sagebrush/grass with 40% vegetative cover density	56		1%	0.56
C	Sagebrush/grass with 40% vegetative cover density	68		3%	2.04
D	Sagebrush/grass with 40% vegetative cover density	76		96%	72.96
Totals =				100%	75.56
CN (weighted) = (total product) / (total area) =		75.56		Use CN =	76
Total Sub-basin Area =				0.576 sq. miles	
Developed Land Use	Soil Name and hydrologic group	CN per Table 702	Area sq. miles	% of Total Area	Product of CN X area
RES 1/3 acre	D	86	0.069	11.98%	10.3
Irr. Pasture (good)	D	80	0.05	8.68%	6.9
Undeveloped	D	76	0.457	79.34%	60.3
				100.00%	
Total Developed Land Use Weighted CN =					78.0

Worksheet 2: Runoff curve number

Project: Evans Creek Block N Hydrology		By: CA		Date: 06/11		
Location: Phase II Sub-Basin E5		Chk:		Date:		
Mark 1	Undeveloped	X	Developed			
1 Runoff curve number (CN)			Note: Sage/Grass CN's per Draft Sparks Manual; Fig. 702			
Soil Name and hydrologic group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connectiod impervious area ratio)	CN 1 per line			Area Mark 1 acres sq. mi. percent	Product of CN X area
		Table 702	Fig. 702	Fig. 2-4		
A	N/A			N/A	0%	N/A
B	Sagebrush/grass with 40% vegetative cover density			56	0%	0
C	Sagebrush/grass with 40% vegetative cover density			68	1%	0.68
D	Sagebrush/grass with 40% vegetative cover density			76	99%	75.24
				Totals =	100%	75.92
CN (weighted) = (total product) / (total area) =		75.92		Use CN =	76	
				Total Sub-basin Area =	0.310	sq. miles
Developed Land Use	Soil Name and hydrologic group	CN per Table 702	Area sq. miles	% of Total Area	Product of CN X area	
RES 1/3 acre	D	86	0.008	2.58%	2.2	
RES 1/3 acre	D	86	0.031	10.00%	8.6	
Irr. Pasture/Park	D	80	0.17	54.84%	43.9	
Undeveloped	D	76	0.101	32.58%	24.8	
				100.00%		
Total Developed Land Use Weighted CN =					80.0	

Appendix C



Nimbus Engineers

3785 Baker Lane, Suite 201 • Reno, NV 89509
Mail: P.O. Box 10220 • Reno, NV 89510
(702) 689-8630

JOB EVANS CREEK 07725-4912

SHEET NO. 1 OF 5

CALCULATED BY CA DATE 5/26/99

CHECKED BY RJ DATE 5/27

SCALE _____

LAG TIME CALCULATIONS (PER DRAFT WASHOE COUNTY DRAINAGE MANUAL)

PHASE II

SUB-BASIN E1 ⇒ AREA = 0.772 MI² < 1 MI²; AVE. SLOPE = 15% > 10%

USE TLAG EQ. 710

$$TLAG = 22.1 K_n (L L_c / S^{1/2})^{1.7}$$

1) DEVELOP K_n FOR PARTIALLY DEVELOPED BASIN

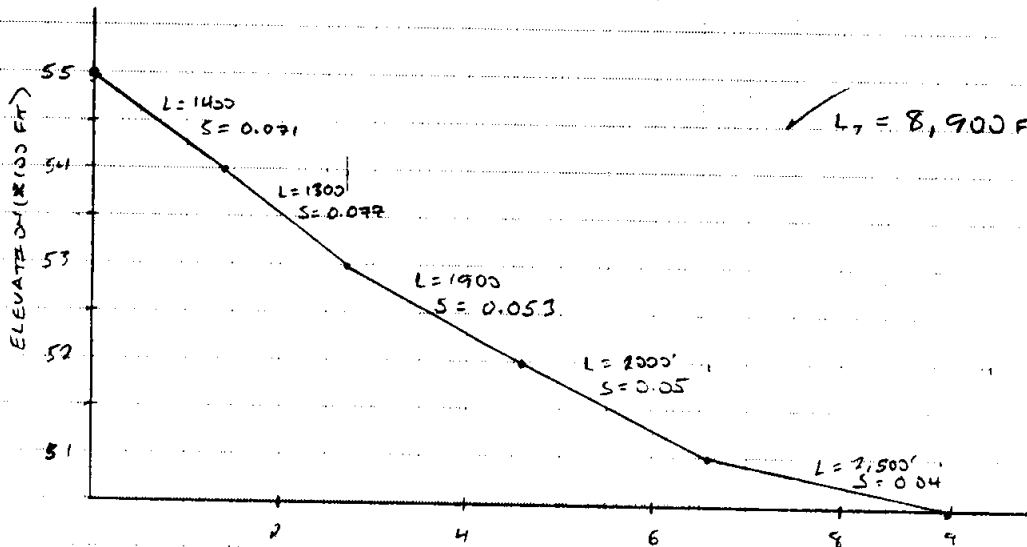
- COMMERCIAL $K_n = 0.05$; %OA = 3.8% ✓
- RESIDENTIAL VS ALRE LOTS; AVE. IMPERVIOUS 30%; $K_n = 0.05$; %OA = 16.4% ✓
- UNDEVELOPED RANGELAND; MIXED GRASS/SHRUB; $K_n = 0.09$; %OA = 83.6% ✓

$$K_n = 0.05(0.038) + 0.05(0.164) + 0.09(0.836)$$

$$K_n = 0.083$$

WEIGHTED K_n FOR
SUB-BASIN

2) DEVELOP AVE. SLOPE (S) OF LONGEST WATERCOURSE



• EQUIVALENT SLOPE; $S_E = \frac{\sum L L_c}{\sum L L_c / S_c^{1/2}}^2 = \left(\frac{8,900'}{\frac{1400}{0.071^{1/2}} + \frac{1300}{0.072^{1/2}} + \frac{1900}{0.053^{1/2}} + \frac{2000}{0.05^{1/2}} + \frac{2500}{0.04^{1/2}}} \right)^2$

$$S_E = 0.05 \text{ FT/FT} = 265 \text{ FT/MI}$$

EQUIVALENT SLOPE
OF LONGEST WATERCOURSE



Nimbus Engineers

3785 Baker Lane, Suite 201 • Reno, NV 89509
Mail: P.O. Box 10220 • Reno, NV 89510
(702) 889-8630

JOB EVANS CREEK 07775-9912
SHEET NO. 2 OF 5
CALCULATED BY CK DATE 5/26/99
CHECKED BY RJJ DATE 5-4-99
SCALE _____

LAG TIME CALCULATIONS (CONT'D)

PHASE II

SUB-BASIN E1

3) DEVELOP LENGTH OF WATERCOURSE TO POINT OPPOSITE BASIN CENTROID
(SEE WORKSHEET MAP DATED MAY 25, 1999)

$$4,000 = 0.76 \text{ mi}$$
$$L_c = 3,400 \text{ FT} = 0.65 \text{ MI} \leftarrow \text{WATERCOURSE CENTROID LENGTH}$$

4) CALCULATE TLAG

$$TLAG = 22.1 (0.085) \left(\frac{(1.7)(0.65)}{(265)^2} \right)^{1/3}$$

$$TLAG = 0.77 \text{ HOURS} \leftarrow \text{LAG TIME FOR BASIN E1}$$

0.74 hr

SUB-BASIN E2 \Rightarrow AREA = 1.2 MI² > 1 MI²; AVE. SLOPE \approx 10%
USE TLAG EQ. 710

1) DEVELOP K_N FOR PARTIALLY DEVELOPED BASIN

- COMMERCIAL $K_N = 0.05$; % A = 4.7% ✓
- INDUSTRIAL $K_N = 0.05$; % A = 3.1% ✓
- RESIDENTIAL: 1/3 ACRE LOTS; 30% AVE. IMPERVIOUS; $K_N = 0.05$, % A = 22% ✓
1 ACRE LOTS; 20% AVE. IMPERVIOUS; $K_N = 0.09$, % A = 3.3% ✓
- UNDEVELOPED RANGELAND; MIXED GRASS/SHRUB; $K_N = 0.09$; % A = 66.9% ✓
67.3%

$$K_N = 0.05(0.047) + 0.05(0.031) + 0.05(0.22) + 0.09(0.033) + 0.09(0.669)$$

$$K_N = 0.078 \leftarrow \text{WEIGHTED } K_N \text{ FOR E2}$$



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JOB EVANS CREEK

07775-9912

SHEET NO. 3

OF 5

CALCULATED BY CA

DATE 5/26/99

CHECKED BY [Signature]

DATE 6-0-00

SCALE _____

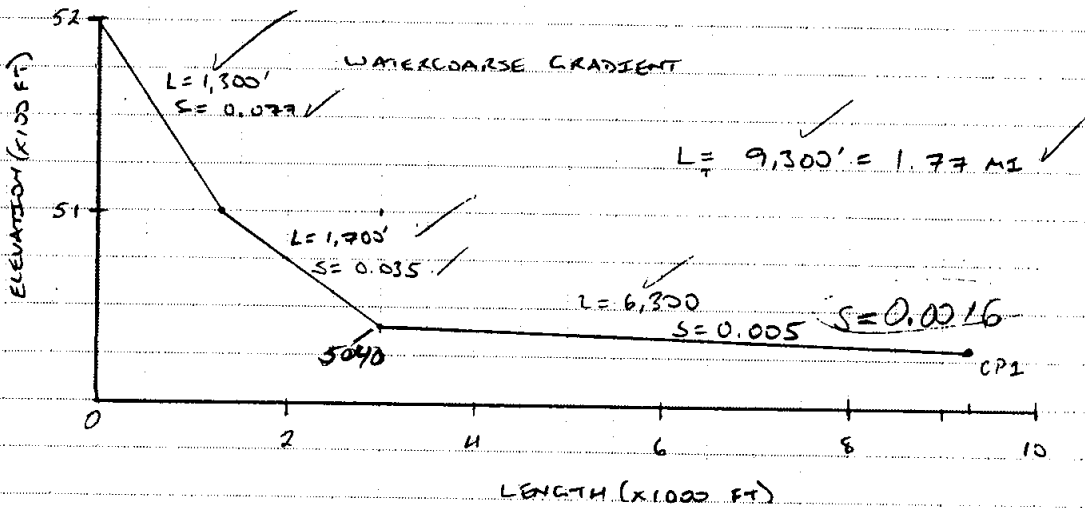
LAG TIME CALCULATIONS

PHASE II

SUB-BASIN E2 (CONT'D)

2) DEVELOP AVE. SLOPE

- LONGEST WATERCOURSE STARTS IN NE CORNER OF E2 AND RUNS TO CPI



$$S_E = \left(\frac{9300}{\frac{1300}{0.0277^{1/2}} + \frac{1700}{0.035} + \frac{6300}{0.005^{1/2}}} \right)^2 \quad \bar{S} = 0.0182 = 96 \frac{\text{FT}}{\text{MI}}$$

$S_E = 0.008 \text{ FT/FT} = 43 \text{ FT/MI}$ ← EQUIVALENT SLOPE OF WATERCOURSE

3) DEVELOP L_c (SEE MAP); $L_c = 4,900' = 0.93 \text{ MI}$

4) CALCULATE TLAG

$$TLAG = 22.1 (0.078) \left(\frac{(1.77)(0.93)}{(43)^{1/2}} \right)^{1.5}$$

$TLAG = 1.1 \text{ HOURS}$ ← LAG TIME FOR E2

$t_{lag} = 0.99 \text{ hrs}$



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SHEET NO. 4

OF 5

CALCULATED BY CA

DATE 5/26/99

CHECKED BY RJJ

DATE 6-4-99

SCALE

PHASE II (CONT'D)

SUB-BASIN E3 ⇒ AREA = 1.51 MI² > 1 MI²; USE TLAG EQ. 710

1) DEVELOP K_N FOR PARTIALLY DEVELOPED BASIN

• RESIDENTIAL: 1/2 ACRE LOTS, 65% AVE. IMPERVIOUS, $K_N = 0.05$
%A = 1.7% ✓

1 ACRE LOTS, 20% AVE. IMPERVIOUS, $K_N = 0.07$, %A = 7.7% ✓

• UNDEVELOPED RANGELAND, MIXED SHRUB/GRASS; $K_N = 0.09$; %A = 9.1% ✓

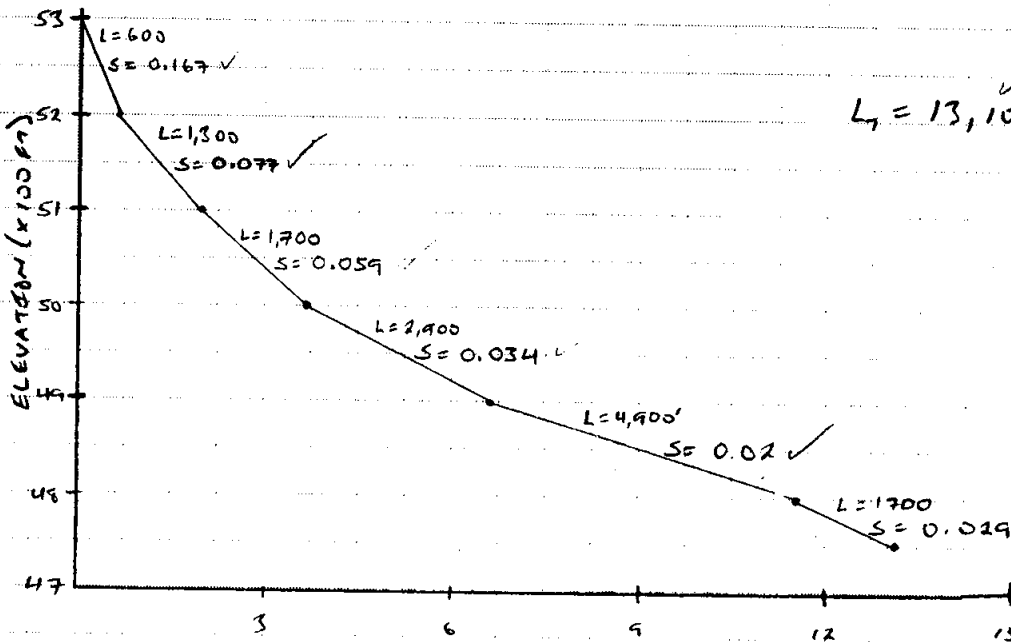
$$K_N = 0.05(0.017) + 0.07(0.073) + 0.09(0.91) =$$

$$K_N = 0.088$$

WEIGHTED K_N FOR E3

2) DEVELOP S_E FOR LONGEST WATERCOURSE IN E3

• LONGEST WATERCOURSE STARTS IN NW CORNER OF E3 AND RUNS TO C/P2



$$L = 13,100' = 2.5 \text{ MI}$$

$$S_E = \left(\frac{\text{LENGTH (X1000 FT)}}{13,100} \right)^2 \left(\frac{600}{0.167^{1/2}} + \frac{1300}{0.077^{1/2}} + \frac{1700}{0.059^{1/2}} + \frac{2900}{0.034^{1/2}} + \frac{4900}{0.02^{1/2}} + \frac{1700}{0.029^{1/2}} \right)^2$$

$$S_E = 0.032 \text{ FT/FT} = 167 \text{ FT/MI}$$

EQUIVALENT SLOPE

$$S = 0.0417 \text{ FT/FT (WEIGHTED)} = 220 \text{ FT/MI}$$

OF WATERCOURSE

$$S = 0.010425 \text{ UNWEIGHTED}$$



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DATE 5/26/99

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DATE 6-4-99

SCALE _____

PHASE II

SUB-BASIN E3 (CONT'D)

- 3) DEVELOP LENGTH OF WATERCOURSE TO POINT OPPOSITE BASIN CENTERED (SEE MAP)

$$L_c = 8,000' = 1.5 \text{ MI}$$

- 4) CALCULATE TLAG

$$TLAG = 22.1 (0.088) \left(\frac{(2.5)(1.5)}{(167)^{1/2}} \right)^{1/3}$$

$$TLAG = 1.3 \text{ HOURS}$$

$$t_{lag} = 1.24 \text{ hr}$$

SUB-BASINS E4 AND E5

- SUB-BASINS E4 AND E5 ARE SAME AS PHASE I SUB-BASINS E3 AND E4, RESPECTIVELY.

$$TLAG E4 = 0.57 \text{ HOURS} \leftarrow$$

LAG TIME FOR E4

$$TLAG E5 = 0.5 \text{ HOURS} \leftarrow$$

LAG TIME FOR E5



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DATE 5/25/99

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DATE 6-1-99

SCALE _____

LAG TIME CALCULATIONS (CONT'D)

PHASE I

SUB-BASIN E3 ⇒ AREA = 0.576 MI² < 1 MI²; AVE. SLOPE > 10%

∴ USE TLAC EQ. 710

$$TLAC = 22.1 K_N (LLC/S^{1/2})^{1/3}$$

1) DEVELOP K_N FOR PARTIALLY DEVELOPED BASIN

- RESIDENTIAL 1/3 ACRE LOTS; 30% AVE. IMPERVIOUS; $K_N = 0.05$; %A = 12%
- AGRICULTURE (IRRIGATED); $K_N = 0.10$; %A = 8.6%
- UNDEVELOPED RANGELAND, MIXED GRASS/SHRUB; $K_N = 0.09$; %A = 79.4%

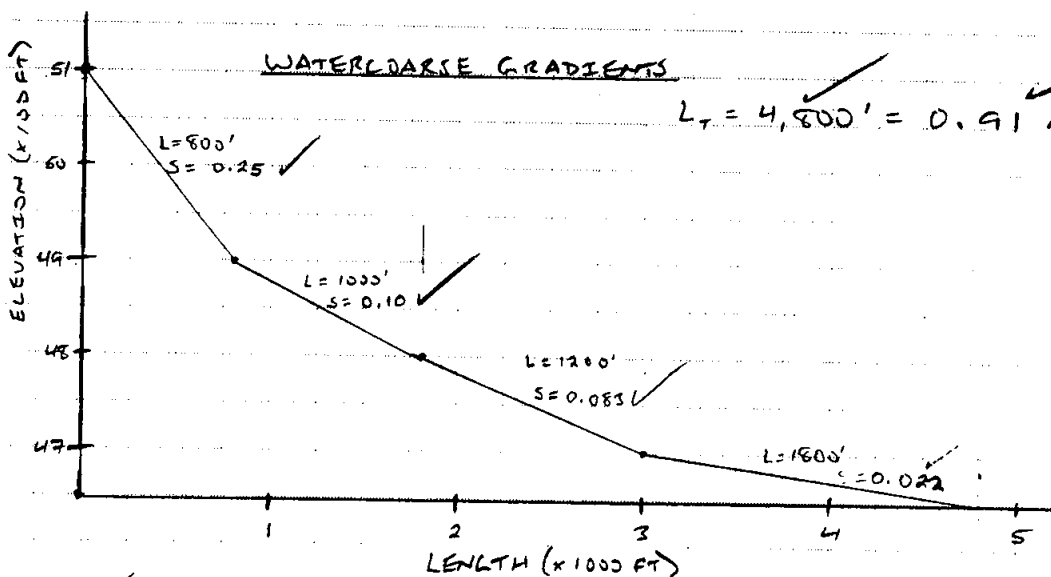
$$K_N = 0.05(0.12) + 0.10(0.086) + 0.09(0.794)$$

$$K_N = 0.086$$

WEIGHTED K_N FOR SUB-BASIN

2) DEVELOPE AVE. SLOPE (S) OF LONGEST WATERCOURSE (FEET/MILE)

- LONGEST WATERCOURSE IN E3 STARTS IN WEST PORTION & RUNS INTO EVANS CREEK (BELOW DITCH FLUME) THEN TO CP3.



$$S_E = \left(\frac{4,800}{\frac{800}{0.25^2} + \frac{1000}{0.10^2} + \frac{1200}{0.083^2} + \frac{1800}{0.022^2}} \right)^2$$

$$S_E = 0.052 \text{ FT/FT} = 273 \text{ FT/MI}$$

EQUIVALENT SLOPE

OF LONGEST WATERCOURSE

$$S_{\text{weighted}} = 0.0415 \text{ FT/FT} = 483$$

$$S_{\text{unweighted}} = 0.1915 \text{ FT/MI} = 483$$



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DATE 6-4-99

SCALE _____

LAG TIME CALCULATIONS (CONT'D)

PHASE I

SUB-BASIN E3 (CONT'D)

- 3) DEVELOP LENGTH OF WATERCOURSE TO POINT OPPOSITE (ENTRIED)
(SEE WORKING MAP)

$$L_c = 2,400' = 0.46 \text{ MI} \checkmark$$

WATERCOURSE LENGTH
TO BASIN ENTRIED

- 4) CALCULATE TLAG

$$TLAG = 22.1 (0.086) \left(\frac{0.91 (0.46)}{(273 \text{ ft/hr})^{1/2}} \right)^{1/3}$$

$$TLAG = 0.57 \text{ HOURS} \checkmark$$

LAG TIME FOR
SUB-BASIN E3

$$t_{lag} = 0.51 \text{ hr} \checkmark$$

SUB-BASIN E4 ⇒ AREA = 0.31 MI² < 1 MI²; AVE. SLOPE < 10%

∴ USE TLAG EQ. 709

$$TLAG = 0.6 t_c \quad \text{WHERE } t_c = t_i + t_o$$

- 1) CALCULATE t_i ⇒ $t_i = \frac{1.8(1.1-R)L^{1/2}}{S^{1/3}}$

• WHERE $R = 0.0132(CN - 0.39)$
 $= 0.0132(78) - 0.39$

CN = 80

$$R = 0.64 \checkmark$$

$$R = 0.64$$

FLOW RUNOFF
COEFFICIENT

- AVERAGE BASIN SLOPE (%); $S = 5.8\% \checkmark$

AVE. BASIN SLOPE (%)
(ESTIMATED)

- SAY $L_o = 500'$

$$t_i = \frac{1.8(1.1-0.64)(500)^{1/2}}{(5.8)^{1/3}} = 10.4 \text{ MIN} \checkmark$$

TIME FOR INITIAL
OVERLAND FLOW

$$t_i = 7.6 \text{ min} \checkmark$$

702.1



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SHEET NO. 6

OF 6

CALCULATED BY CA

DATE 5/26/99

CHECKED BY [Signature]

DATE 6-7-99

SCALE _____

PHASE I

SUB-BASIN E4 (CONT'D)

2) CALCULATE TRAVEL TIME (t_L)

SAY THREE REACHES IN E4

• FROM WESTERLY PORTION E4 TO HIGHLAND DITCH

✓ 1. $S = 5.6\%$ AND ALLUVIAL FAN WESTERN Mtn. REACHES;
USING FIG. 7.01

✓ $V = 2.5$ FPS ✓ $L = 1,500$ FT ∴ $t_L = 600s = 10$ MIN

2. ✓ $S = 4.0\%$ AND SHORT GRASSED PASTURE

$V = 1.8$ FT/S ✓ $L = 1,500$ FT ∴ $t_L = 13.9$ MIN
(Handwritten: 1.4, 17.7 min)

3. $S = 2\%$ AND EVANS CREEK CHANNEL; SAY $R_m = 0.33$; $n = 0.045$

$V = \frac{1.49}{0.045} (0.33)^{2/3} (2000\text{FT})^{1/2}$ $L = 2000'$

✓ $V = 2.2$ FPS ∴ $t_L = \frac{2000'}{2.2 \text{ FPS} \times \frac{1}{60}} = 15.2$ MIN
(Handwritten: OK)

• t_L FOR SUB-BASIN E4

$t_L = 10 + 13.9 + 15.2 = 39$ MIN ≈ 40 MIN = 0.67 HR
(Handwritten: 17.9)

3) CALCULATE TLAG

$TLAG = 0.6 t_L = 0.6 (t_{L1} + t_{L2}) = 0.6 (10.4 \text{ MIN} + 40 \text{ MIN})$
(Handwritten: 9.6)

✓ $TLAG = 0.5$ HOURS ← LAG TIME FOR SUB-BASIN E4

$t_{lag} = 0.53$ hr

OK

Appendix D

5-YEAR, 24 HOUR

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID      PHASE II SUB-AREAS
2         ID
3         ID      EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID      5 YEAR 24 HOUR EVENT                 DATE : MAY 1999
5         ID      NIMBUS ENGINEERS, RENO, NEVADA         FILE NAME: 2EVNSS.DAT
6         ID
7         ID
8         ID      THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID      *****
10        ID      GONI CANYON                            Nimbus Job #: 9606      *
11        ID      Prepared by Nimbus Engineers           Date: August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID      *****
14        ID
15        ID      The 5-yr PH card rainfall depths were determined from the
16        ID      Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID      Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID      events and the methodology per the Draft Washoe County Hydrologic
19        ID      Criteria and Design Manual.
20        ID
21        ID      *****
22        ID      THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
23        ID      EXISTING CONDITIONS (PLAN 2)                                     *
24        ID      *****
25        IT      5 01MAY99    0005    288
26        IO      4      0
27        JP      2
          * DEPTH AREA REDUCTION FACTOR FROM NOAA
28        JR      PREC    0.99
          *

29        KK      E1 SUBBASIN E1
30        BA      0.772
31        PH              0.18    0.32    0.54    0.73    0.87    1.17    1.45    1.72
32        LS              76
33        UD      0.79
          * ZW A=5-YR C=FLOW F=E1FLOW
          *

34        KK      E2 SUB-BASIN E2
35        BA      1.2
36        PH              0.18    0.32    0.54    0.70    0.82    1.07    1.32    1.57
37        LS              77
38        UD      0.99
          * ZW A=5-YR C=FLOW F=E2FLOW
          *

39        KK      CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
40        HC      2
          * ZW A=5-YR C=FLOW F=E1+E2
          *
    
```

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
41	KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
42	RD 10800 0.026 0.07 TRAP 6 2
	*
43	KK E3 SUBBASIN E3
44	BA 1.51
45	PH 0.18 0.32 0.54 0.70 0.82 1.07 1.34 1.61
46	LS 81
47	UD 1.24
	* ZW A=5-YR C=FLOW F=E3FLOW
	*
48	KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
49	HC 2
	* ZW A=5-YR C=FLOW F=E1+E2+E3
	*
50	KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
	* KO 2
	* KP 1
	* KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.
	* RS 1 STOR 0
	* SA .1354 .9137 1.954 3.63 3.71 3.79 3.87 3.95 5.12 6.
	* SA 7.97 8.92 9.93 10.18 10.45 10.94 11.55 12.67 14.15 15.
	* SE 765 775 785 795.89 796.33 796.76 797.2 797.64 803.16 808.
	* SE814.22 816.98 819.74 820.36 820.97 822.07 823.42 825.87 828.94 832
	* SQ 0 0 0 0 13.46 38.09 69.97 107.73 114.35 120.
	* SQ126.56 129.43 132.24 218.16 401.01 881.31 1708.7 3654.6 6920.1 11031
	* ZW A=5-YR C=FLOW F=DAMFLOW
	* ZW C=STOR F=DAMSTOR
	* ZW C=STAGE F=DAMSTAGE
	*
51	KP 1
52	KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa
53	RS 1 STOR 0
54	SA 1.18 1.21 1.23 1.27 1.32 1.41 1.50 1.59 1.70 1.91
55	SA 2.83 3.91 3.97 4.72 6.68 9.07 10.37 11.09 12.75 15.70
56	SE 775.4 775.7 776.0 776.4 777.0 778.0 779.0 780.0 781.0 783.0
57	SE 790 796 796.3 800.4 808.69 816.98 820.36 822.10 825.87 832.0
58	SQ 5.4 12.6 21.4 35 40.8 49.9 57.5 64.3 70.4 81.3
59	SQ 92.1 95.43 112.2 134 145 155.4 244.5 907.5 3680.8 11060
60	ZW A=5-YR C=FLOW F=30CMP
61	ZW C=STOR F=30CMP
62	ZW C=STAGE F=30CMP
	*
	* KO 2
63	KP 2
64	KM EXISTING CONDITIONS (NO RESERVOIR)
65	RN
66	ZW A=5-YR B=EXIST C=FLOW F=EXISTFLOW
	*

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
67	KK RCH3 ROUTE TO CP3
68	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
69	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
70	RD 4200 0.023 0.09 TRAP 6 2
	*
71	KK E4 SUBBASIN E4
72	BA 0.58
73	PH 0.18 0.32 0.54 0.69 0.80 1.03 1.27 1.5
74	LS 78
75	UD 0.57
	* ZW A=5-YR C=FLOW F=E4FLOW
	*
76	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
77	HC 2
78	ZW A=5-YR C=FLOW F=E4+CP2
	*
79	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
80	RD 3400 0.019 0.045 TRAP 6 2
	*
81	KK E5 SUB-BASIN E5
82	BA 0.31
83	PH 0.17 0.32 0.53 0.69 0.80 1.05 1.23 1.41
84	LS 80
85	UD 0.5
	* ZW A=5-YR C=FLOW F=E5FLOW
	*
86	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
87	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
88	HC 2
	* <i>ZW A=5-yR C=FLOW F=E5 SIERRA ST</i>
89	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
90	KM STORAGE ROUTING AT SIERRA STREET
91	RS 1 STOR 0
92	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
93	SE 587 594 596 598 600 602 604
94	SQ 0 193 211 229 245 637 1000
95	ZW A=5-YR C=FLOW F=SIERRA STREET
	*
96	ZZ

SCHMATIC DIAGRAM OF STREAM NETWORK

PUT	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
LINE	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
29	E1	
	.	
34	.	E2
	.	.
	.	.
39	CP1.....	
	V	
	V	
41	RCH1	
	.	
43	.	E3
	.	.
	.	.
48	CP2.....	
	V	
	V	
50	DAM1	
	V	
	V	
67	RCH3	
	.	
71	.	E4
	.	.
	.	.
76	CP3.....	
	V	
	V	
79	RCH3	
	.	
81	.	E5
	.	.
	.	.
86	CP4.....	
	V	
	V	
89	SIERRA	

**) RUNOFF ALSO COMPUTED AT THIS LOCATION


```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                   *
*
* RUN DATE 06/29/1999 TIME 16:41:32 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS    *
*   HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET             *
*   DAVIS, CALIFORNIA 95616      *
*   (916) 756-1104               *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
5 YEAR 24 HOUR EVENT              DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA     FILE NAME: 2EVNSS.DAT

```

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMBUS

```

*****
GONI CANYON                        Nimbus Job #: 9606      *
Prepared by Nimbus Engineers        Date: August 1996   *
                                   File Name: GONI-5.DAT  *
*****

```

The 5-yr PH card rainfall depths were determined from the Precipitation Frequency Study of the United States, NOAA Atlas 14, Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation events and the methodology per the Draft Washoe County Hydrologic Criteria and Design Manual.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND *
EXISTING CONDITIONS (PLAN 2)                               *
*****

```

26 IO

OUTPUT CONTROL VARIABLES

```

IPRNT      4  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL     0.  HYDROGRAPH PLOT SCALE

```

IT

HYDROGRAPH TIME DATA

```

NMIN       5  MINUTES IN COMPUTATION INTERVAL
IDATE     1MAY99  STARTING DATE
ITIME     0005  STARTING TIME
NQ        288  NUMBER OF HYDROGRAPH ORDINATES
NDDATE    2MAY99  ENDING DATE
NDTIME    0000  ENDING TIME
ICENT     19  CENTURY MARK

```

```

COMPUTATION INTERVAL   .08 HOURS
TOTAL TIME BASE       23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA          SQUARE MILES
PRECIPITATION DEPTH    INCHES
LENGTH, ELEVATION      FEET
FLOW                   CUBIC FEET PER SECOND
STORAGE VOLUME         ACRE-FEET
SURFACE AREA           ACRES
TEMPERATURE            DEGREES FAHRENHEIT

```

JP

MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 .99

 * *
 * *
 * *

29 KK * E1 * SUBBASIN E1

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
 TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.18	.32	.54	.73	.87	1.17	1.45	1.72	.00	.00	.00	.00

STORM AREA = .77

32 LS SCS LOSS RATE

STRTL	.63	INITIAL ABSTRACTION
CRVNBR	76.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .79 LAG

UNIT HYDROGRAPH
 49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

*** **

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

 * *
 * *
 * *

4 KK * E2 * SUB-BASIN E2

SUBBASIN RUNOFF DATA

35 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

36 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.18 .32 .54 .70 .82 1.07 1.32 1.57 .00 .00 .00 .00

STORM AREA = 1.20

37 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

38 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

* *
39 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* *

40 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

* *
41 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
* *

HYDROGRAPH ROUTING DATA

42 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 6.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

* *** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

13 KK

* *
 * E3 * SUBBASIN E3
 * *

SUBBASIN RUNOFF DATA

44 BA

SUBBASIN CHARACTERISTICS

TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

45 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.18	.32	.54	.70	.82	1.07	1.34	1.61	.00	.00	.00	.00

STORM AREA = 1.51

46 LS

SCS LOSS RATE

STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

7 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG 1.24 LAG

UNIT HYDROGRAPH

76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.
7.	6.	6.	5.	5.	4.	4.	3.	3.	3.
2.	2.	1.	1.	1.	0.				

** *** **

 * *
 48 KK * CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
 * *

49 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

 * *
 50 KK * DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 * *

51 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

53 RS	STORAGE ROUTING											
	NSTPS	1	NUMBER OF SUBREACHES									
	ITYP	STOR	TYPE OF INITIAL CONDITION									
	RSVRIC	.00	INITIAL CONDITION									
	X	.00	WORKING R AND D COEFFICIENT									
54 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9	
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7	
56 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00	
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00	
58 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.	
		92.	95.	112.	134.	145.	155.	245.	908.	3681.	11060.	

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	.36	.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

-----DSS---ZOPEN: Version: 6-EA; New File Opened

Unit: 71, File: 2EVNS5.DSS

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/DAM1/FLOW/01MAY1999/5MIN/30CMP/

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/DAM1/STOR/01MAY1999/5MIN/30CMP/

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/DAM1/STAGE/01MAY1999/5MIN/30CMP/

* *** **

63 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

65 RN NO ROUTING

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/

-----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

* *

67 KK RCH3 ROUTE TO CP3

* *

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

70 RD MUSKINGUM-CUNGE CHANNEL ROUTING
L 4200. CHANNEL LENGTH
S .0230 SLOPE
N .090 CHANNEL ROUGHNESS COEFFICIENT
CA .00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 6.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

* *** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* *

71 KK E4 SUBBASIN E4

* *

SUBBASIN RUNOFF DATA

72 BA SUBBASIN CHARACTERISTICS
TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

73 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.18	.32	.54	.69	.80	1.03	1.27	1.50	.00	.00	.00	.00

STORM AREA = .58

74 LS

SCS LOSS RATE

STRTL .56 INITIAL ABSTRACTION
 CRVNBR 78.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

75 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .57 LAG

UNIT HYDROGRAPH

36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

* *** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

*** **

76 KK

 * *
 * CP3 *
 * *

COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3

77 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 1: /5-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
 -----DSS---ZWRITE Unit 71; Vers. 1: /5-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

*** **

79 KK

 * *
 * RCH3 *
 * *

ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S

HYDROGRAPH ROUTING DATA

80 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 3400. CHANNEL LENGTH
 S .0190 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE

WD 6.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* *
81 KK * E5 * SUB-BASIN E5
* *

SUBBASIN RUNOFF DATA

82 BA SUBBASIN CHARACTERISTICS
TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

83 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.17 .32 .53 .69 .80 1.05 1.23 1.41 .00 .00 .00 .00

STORM AREA = .31

84 LS SCS LOSS RATE
STRTL .50 INITIAL ABSTRACTION
CRVNBR 80.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

85 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .50 LAG

UNIT HYDROGRAPH
32 END-OF-PERIOD ORDINATES
19. 55. 113. 189. 248. 274. 274. 251. 219. 175.
129. 100. 77. 62. 48. 37. 29. 23. 18. 14.
11. 8. 6. 5. 4. 3. 3. 2. 2. 1.
1. 0.

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

* *
86 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
* *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

88 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

89 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

91 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

92 SA	AREA	.0	.5	.8	1.2	1.9	2.6	3.1
93 SE	ELEVATION	587.00	594.00	596.00	598.00	600.00	602.00	604.00
94 SQ	DISCHARGE	0.	193.	211.	229.	245.	637.	1000.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	1.19	2.45	4.40	7.45	11.90	17.61
ELEVATION	587.00	594.00	596.00	598.00	600.00	602.00	604.00

---DSS---ZWRITE Unit 71; Vers. 1: /5-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 1: /5-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	31.
				TIME	13.08
			2	FLOW	31.
				TIME	13.08
HYDROGRAPH AT	E2	1.20	1	FLOW	38.
				TIME	13.33
			2	FLOW	38.
				TIME	13.33
COMBINED AT	CP1	1.97	1	FLOW	67.
				TIME	13.17
			2	FLOW	67.
				TIME	13.17
ROUTED TO	RCH1	1.97	1	FLOW	68.
				TIME	13.75
			2	FLOW	68.
				TIME	13.75
HYDROGRAPH AT	E3	1.51	1	FLOW	77.
				TIME	13.50
			2	FLOW	77.
				TIME	13.50
COMBINED AT	CP2	3.48	1	FLOW	143.
				TIME	13.75
			2	FLOW	143.
				TIME	13.75
ROUTED TO	DAM1	3.48	1	FLOW	79.
				TIME	15.75
			2	FLOW	143.
				TIME	13.75
** PEAK STAGES IN FEET **					
			1	STAGE	782.56
				TIME	15.75
			2	STAGE	.00
				TIME	.00
ROUTED TO	RCH3	3.48	1	FLOW	79.
				TIME	16.00
			2	FLOW	146.
				TIME	13.92
HYDROGRAPH AT	E4	.58	1	FLOW	25.
				TIME	12.75
			2	FLOW	25.
				TIME	12.75
COMBINED AT	CP3	4.06	1	FLOW	86.
				TIME	15.50
			2	FLOW	159.
				TIME	13.92

ROUTED TO	RCH3	4.06	1	FLOW	86.
				TIME	15.58
			2	FLOW	158.
				TIME	14.08

DROGRAPH AT	E5	.31	1	FLOW	17.
				TIME	12.58
			2	FLOW	17.
				TIME	12.58

COMBINED AT	CP4	4.37	1	FLOW	91.
				TIME	15.25
			2	FLOW	166.
				TIME	13.83

ROUTED TO	SIERRA	4.37	1	FLOW	91.
				TIME	15.33
			2	FLOW	163.
				TIME	14.08

** PEAK STAGES IN FEET **

1	STAGE	590.30
	TIME	15.33
2	STAGE	592.90
	TIME	14.08

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	4.75	69.94	812.25	.23	5.00	67.52	825.00	.23
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2495E+02 EXCESS= .0000E+00 OUTFLOW= .2398E+02 BASIN STORAGE= .1157E+01 PERCENT ERROR= -.7

FOR PLAN = 2 RATIO= .00

RCH1	MANE	4.75	69.94	812.25	.23	5.00	67.52	825.00	.23
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2495E+02 EXCESS= .0000E+00 OUTFLOW= .2398E+02 BASIN STORAGE= .1157E+01 PERCENT ERROR= -.7

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	78.88	960.00	.30	5.00	78.88	960.00	.30
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5647E+02 EXCESS= .0000E+00 OUTFLOW= .5587E+02 BASIN STORAGE= .6110E+00 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	145.69	835.00	.27	5.00	145.69	835.00	.27
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5186E+02 EXCESS= .0000E+00 OUTFLOW= .5104E+02 BASIN STORAGE= .9430E+00 PERCENT ERROR= -.2

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	86.18	935.00	.29	5.00	86.18	935.00	.29
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6270E+02 EXCESS= .0000E+00 OUTFLOW= .6233E+02 BASIN STORAGE= .3741E+00 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	158.03	845.00	.26	5.00	158.03	845.00	.26
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5787E+02 EXCESS= .0000E+00 OUTFLOW= .5740E+02 BASIN STORAGE= .5472E+00 PERCENT ERROR= -.1

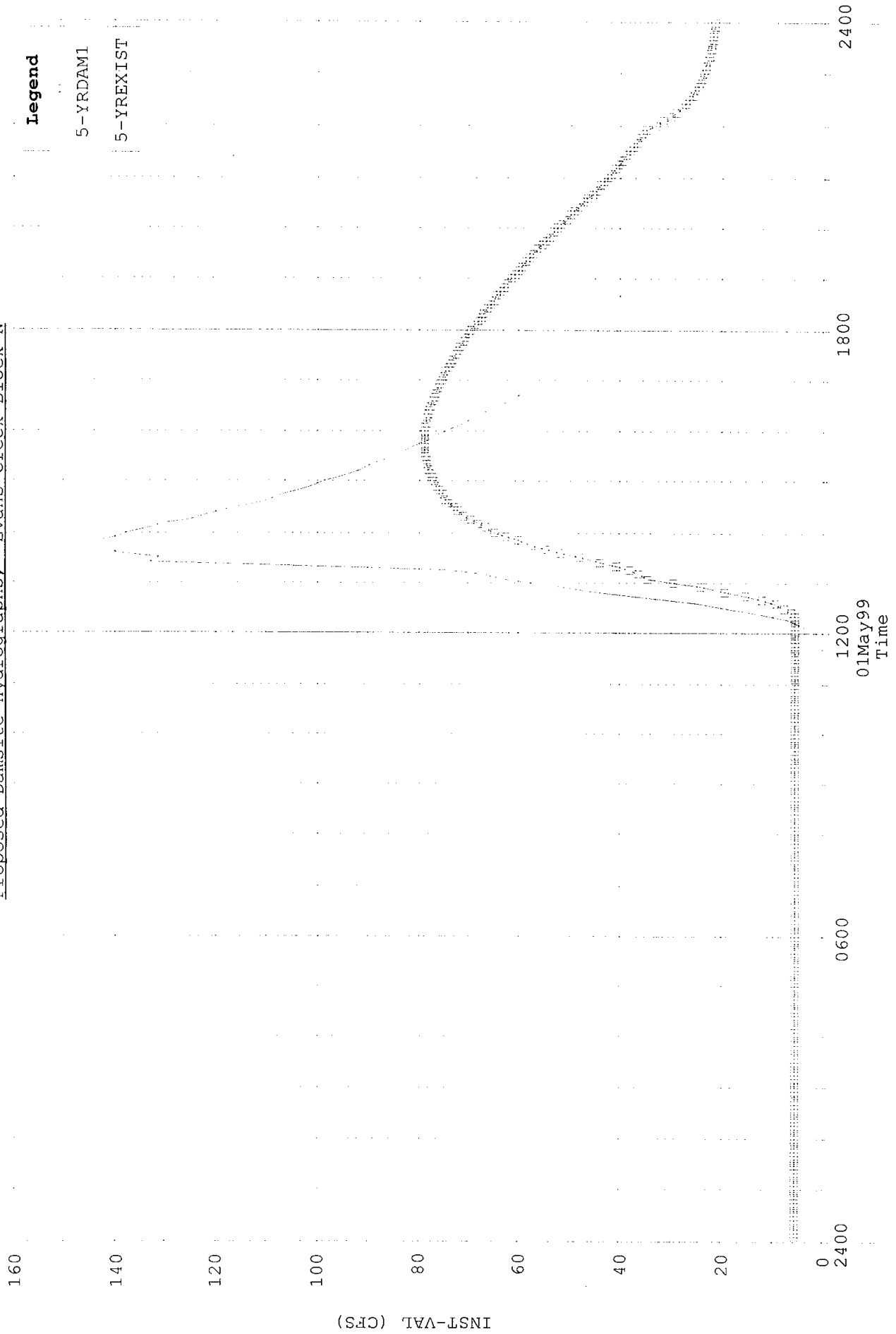
* NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EVNS5.DSS

Pointer Utilization: .25
 Number of Records: 10
 File Size: 24.1 Kbytes
 Percent Inactive: .00

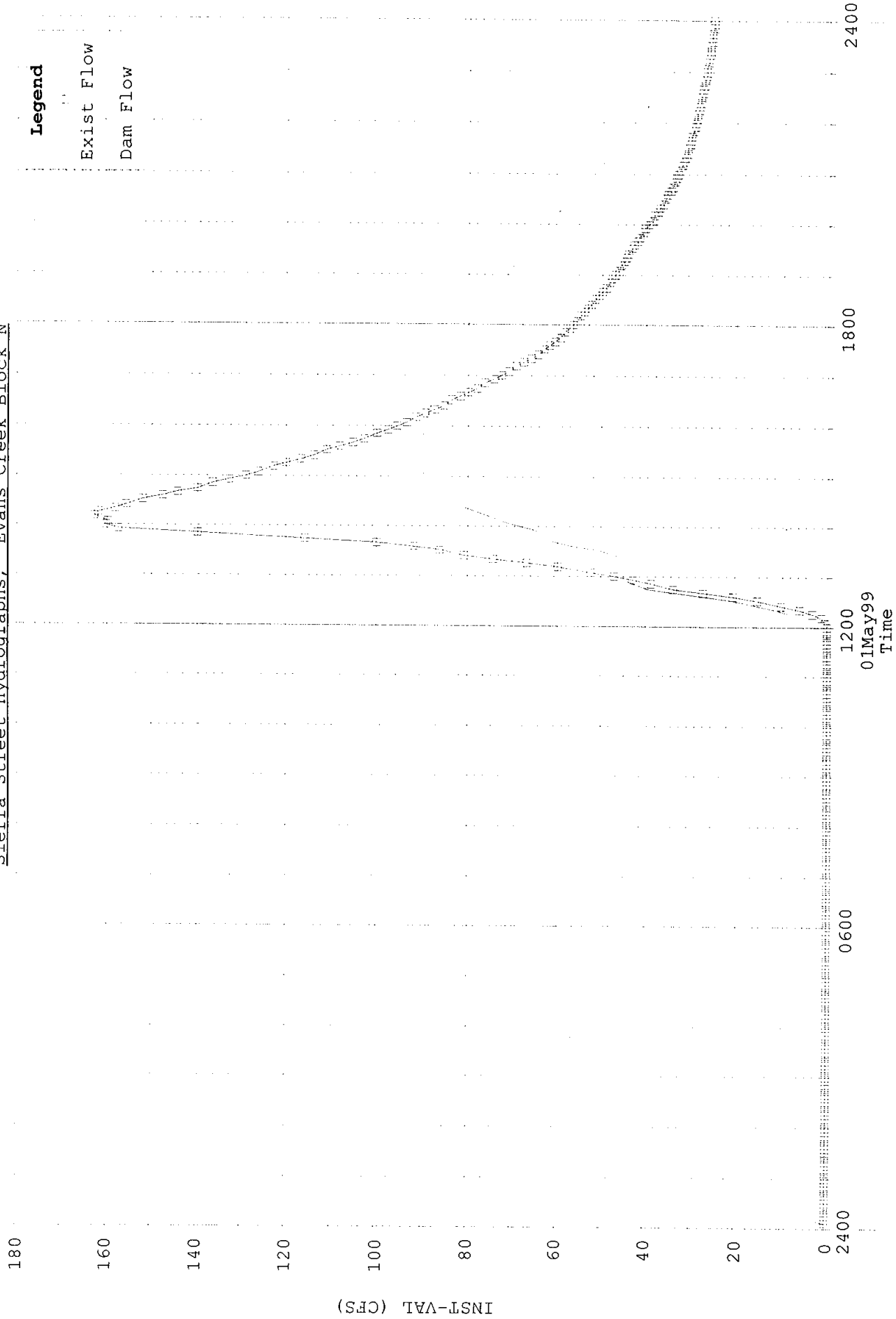
5-YR DAM1, EXIST FLOW

Proposed Damsite Hydrographs; Evans Creek Block N



5-YR SIERRA FLOW

Sierra Street Hydrographs: Evans Creek Block N



10-YEAR, 24 HOUR

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID      PHASE II SUB-AREAS
2         ID
3         ID      EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID      10 YEAR 24 HOUR EVENT                DATE : MAY 1999
5         ID      NIMBUS ENGINEERS, RENO, NEVADA         FILE NAME: 2EVN10.DAT
6         ID
7         ID
8         ID      THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID      *****
10        ID      GONI CANYON                            Nimbus Job #: 9606      *
11        ID      Prepared by Nimbus Engineers           Date: August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID      *****
14        ID
15        ID      The 10-yr PH card depths were determined from the
16        ID      Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID      Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID      events and the methodology per the Draft Washoe County Hydrologic
19        ID      Criteria and Design Manual.
20        ID
21        ID      *****
22        ID      THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
23        ID      EXISTING CONDITIONS (PLAN 2)                                     *
24        ID      *****
25        IT      5 01MAY99    0005    288
26        IO      4      0
27        JP      2
28        *      DEPTH AREA REDUCTION FACTOR FROM NOAA
29        JR      PREC    0.99
30        *
31        KK      E1 SUBBASIN E1
32        BA      0.772
33        PH      0.23  0.41  0.69  0.89  1.05  1.37  1.69  2.01
34        LS      76
35        UD      0.79
36        *      ZW A=10-YR C=FLOW F=E1FLOW
37        *
38        KK      E2 SUBBASIN E2
39        BA      1.2
40        PH      0.23  0.41  0.69  0.86  0.98  1.25  1.55  1.85
41        LS      77
42        UD      0.99
43        *      ZW A=10-YR C=FLOW F=E2FLOW
44        *
45        KK      CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
46        *      KO      2
47        HC      2
48        ZW      A=10-YR C=FLOW F=E1+E2
49        *

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

42 KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

43 RD 10800 0.026 0.07 TRAP 15 2

*

44 KK E3 SUB-BASIN E3

45 BA 1.51

46 PH 0.23 0.41 0.69 0.86 0.98 1.25 1.57 1.89

47 LS 81

48 UD 1.24

* ZW A=10-YR C=FLOW F=E3FLOW

*

49 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3

50 HC 2

* ZW A=10-YR C=FLOW F=E1+E2+E3

*

51 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS

* KO 2

* KP 1

* KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.

* RS 1 STOR 0

* SA .1354 .9137 1.954 3.63 3.71 3.79 3.87 3.95 5.12 6.

* SA 7.97 8.92 9.93 10.18 10.45 10.94 11.55 12.67 14.15 15.

* SE 765 775 785 795.89 796.33 796.76 797.2 797.64 803.16 808.

* SE814.22 816.98 819.74 820.36 820.97 822.07 823.42 825.87 828.94 832

* SQ 0 0 0 0 13.46 38.09 69.97 107.73 114.35 120.

* SQ126.56 129.43 132.24 218.16 401.01 881.31 1708.7 3654.6 6920.1 11031

* ZW A=10-YR C=FLOW F=DAMFLOW

* ZW C=STOR F=DAMSTOR

* ZW C=STAGE F=DAMSTAGE

*

52 KP 1

53 KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

54 RS 1 STOR 0

55 SA 1.18 1.21 1.23 1.27 1.32 1.41 1.50 1.59 1.70 1.91

56 SA 2.83 3.91 3.97 4.72 6.68 9.07 10.37 11.09 12.75 15.70

57 SE 775.4 775.7 776.0 776.4 777.0 778.0 779.0 780.0 781.0 783.0

58 SE 790 796 796.3 800.4 808.69 816.98 820.36 822.10 825.87 832.0

59 SQ 5.4 12.6 21.4 35 40.8 49.9 57.5 64.3 70.4 81.3

60 SQ 92.1 95.43 112.2 134 145 155.4 244.5 3680.8 11060

61 ZW A=10-YR C=FLOW F=30CMP

62 ZW C=STOR F=30CMP

63 ZW C=STAGE F=30CMP

*

* KO 2

64 KP 2

65 KM EXISTING CONDITIONS (NO RESERVOIR)

66 RN

67 ZW A=10-YR B=EXIST C=FLOW F=EXISTFLOW

*

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
68	KK RCH3 ROUTE TO CP3
69	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
70	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
71	RD 4200 0.023 0.09 TRAP 20 2
	*
72	KK E4 SUBBASIN E4
73	BA 0.58
74	PH 0.23 0.41 0.69 0.84 0.96 1.20 1.48 1.76
75	LS 78
76	UD 0.57
	* ZW A=10-YR C=FLOW F=E4FLOW
	*
77	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
78	HC 2
79	ZW A=10-YR C=FLOW F=E4+CP2
	*
80	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
81	RD 3400 0.019 0.045 TRAP 20 2
	*
82	KK E5 SUB-BASIN E5
83	BA 0.31
84	PH 0.22 0.40 0.67 0.84 0.96 1.23 1.44 1.65
85	LS 80
86	UD 0.5
	* ZW A=10-YR C=FLOW F=E5FLOW
	*
87	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
88	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
89	HC 2
	* ZW A=10-YR C=FLOW F=SIERRA STREET
	*
90	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
91	KM STORAGE ROUTING AT SIERRA STREET
92	RS 1 STOR 0
93	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
94	SE 587 594 596 598 600 602 604
95	SQ 0 193 211 229 245 637 1000
96	ZW A=10-YR C=FLOW F=SIERRA STREET
	*
97	ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

PUT	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
INE	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
NO.		
29	E1	
	.	
34	.	E2
	.	.
39	CP1.....	
	V	
	V	
42	RCH1	
	.	
44	.	E3
	.	.
49	CP2.....	
	V	
	V	
51	DAM1	
	V	
	V	
68	RCH3	
	.	
72	.	E4
	.	.
77	CP3.....	
	V	
	V	
80	RCH3	
	.	
82	.	E5
	.	.
87	CP4.....	
	V	
	V	
90	SIERRA	

***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                   *
*
* RUN DATE 06/29/1999 TIME 16:09:29 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET          *
*   DAVIS, CALIFORNIA 95616    *
*   (916) 756-1104             *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
10 YEAR 24 HOUR EVENT              DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA      FILE NAME: 2EVN10.DAT

```

```

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
*****
GONI CANYON                        Nimbus Job #: 9606      *
Prepared by Nimbus Engineers        Date: August 1996   *
                                     File Name: GONI-5.DAT *
*****

```

The 10-yr PH card depths were determined from the Precipitation Frequency Study of the United States, NOAA Atlas 14, Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation events and the methodology per the Draft Washoe County Hydrologic Criteria and Design Manual.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
EXISTING CONDITIONS (PLAN 2)                                     *
*****

```

```

26 IO      OUTPUT CONTROL VARIABLES
          IPRNT      4  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL     0.  HYDROGRAPH PLOT SCALE

```

```

IT      HYDROGRAPH TIME DATA
          NMIN      5  MINUTES IN COMPUTATION INTERVAL
          IDATE     1MAY99  STARTING DATE
          ITIME     0005  STARTING TIME
          NQ        288  NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    2MAY99  ENDING DATE
          NDTIME    0000  ENDING TIME
          ICENT     19  CENTURY MARK

          COMPUTATION INTERVAL      .08 HOURS
          TOTAL TIME BASE           23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

```

JP      MULTI-PLAN OPTION

```

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 .99

 * *
 29 KK * E1 * SUBBASIN E1
 * *

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
 TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.23	.41	.69	.89	1.05	1.37	1.69	2.01	.00	.00	.00	.00

STORM AREA = .77

32 LS SCS LOSS RATE

STRTL	.63	INITIAL ABSTRACTION
CRVNBR	76.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .79 LAG

UNIT HYDROGRAPH
 49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

* *** **

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

 * *
 34 KK * E2 * SUBBASIN E2
 * *

SUBBASIN RUNOFF DATA

35 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

36 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.23 .41 .69 .86 .98 1.25 1.55 1.85 .00 .00 .00 .00

STORM AREA = 1.20

37 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

38 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH

61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

39 KK *****
* *
* CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* *

40 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened
Unit: 71, File: 2EVN10.DSS
---DSS---ZWRITE Unit 71; Vers. 2: /10-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/CP1/FLOW/01MAY1999/5MIN//

42 KK *****
* *
* RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

* *

HYDROGRAPH ROUTING DATA

43 RD MUSKINGUM-CUNGE CHANNEL ROUTING
L 10800. CHANNEL LENGTH
S .0260 SLOPE
N .070 CHANNEL ROUGHNESS COEFFICIENT
CA .00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 15.00 BOTTOM WIDTH OR DIAMETER
Z .00 SIDE SLOPE

** *** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

* *
* E3 * SUB-BASIN E3
* *

44 KK

SUBBASIN RUNOFF DATA

45 BA SUBBASIN CHARACTERISTICS
TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

46 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.23 .41 .69 .86 .98 1.25 1.57 1.89 .00 .00 .00 .00

STORM AREA = 1.51

47 LS SCS LOSS RATE
STRTL .47 INITIAL ABSTRACTION
CRVNBR 81.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

48 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 1.24 LAG

UNIT HYDROGRAPH
76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.
7.	6.	6.	5.	5.	4.	4.	3.	3.	3.
2.	2.	1.	1.	1.	0.				

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

49 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3

50 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

51 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS

52 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

54 RS STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVR IC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

55 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7

57 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

59 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.
		92.	95.	112.	134.	145.	155.	245.	908.	3681.	11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	.36	.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/DAM1/FLOW/01MAY1999/5MIN/30CMP/
-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/DAM1/STOR/01MAY1999/5MIN/30CMP/
-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/DAM1/STAGE/01MAY1999/5MIN/30CMP/

*** **

64 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

66 RN NO ROUTING

-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
-----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

* *
68 KK * RCH3 * ROUTE TO CP3
* *

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

71 RD MUSKINGUM-CUNGE CHANNEL ROUTING
L 4200. CHANNEL LENGTH
S .0230 SLOPE
N .090 CHANNEL ROUGHNESS COEFFICIENT
CA .00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 20.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* *
72 KK * E4 * SUBBASIN E4
* *

SUBBASIN RUNOFF DATA *

73 BA SUBBASIN CHARACTERISTICS
TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

74 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

	HYDRO-35	TP-40	TP-49								
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.23	.41	.69	.84	.96	1.20	1.48	1.76	.00	.00	.00	.00

STORM AREA = .58

75 LS

SCS LOSS RATE

STRTL .56 INITIAL ABSTRACTION
 CRVNR 78.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

76 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .57 LAG

UNIT HYDROGRAPH

36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

77 KK

 * *
 * CP3 *
 * *

COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3

78 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 2: /10-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
 ---DSS---ZWRITE Unit 71; Vers. 2: /10-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

80 KK

 * *
 * RCH3 *
 * *

ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S

HYDROGRAPH ROUTING DATA

81 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 3400. CHANNEL LENGTH
 S .0190 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT

CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

 * *
 82 KK * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

83 BA SUBBASIN CHARACTERISTICS
 TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

84 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.22	.40	.67	.84	.96	1.23	1.44	1.65	.00	.00	.00	.00

STORM AREA = .31

85 LS SCS LOSS RATE

STRTL	.50	INITIAL ABSTRACTION
CRVNBR	80.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

86 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.50	LAG
------	-----	-----

UNIT HYDROGRAPH
 32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

 * *

87 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH

* *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

89 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

90 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET

* *

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

92 RS STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

93 SA AREA .0 .5 .8 1.2 1.9 2.6 3.1

94 SE ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

95 SQ DISCHARGE 0. 193. 211. 229. 245. 637. 1000.

COMPUTED STORAGE-ELEVATION DATA

STORAGE .00 1.19 2.45 4.40 7.45 11.90 17.61
ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

----DSS---ZWRITE Unit 71; Vers. 2: /10-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

*** **

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 2: /10-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	57.
				TIME	12.92
			2	FLOW	57.
				TIME	12.92
HYDROGRAPH AT	E2	1.20	1	FLOW	71.
				TIME	13.17
			2	FLOW	71.
				TIME	13.17
2 COMBINED AT	CP1	1.97	1	FLOW	125.
				TIME	13.08
			2	FLOW	125.
				TIME	13.08
ROUTED TO	RCH1	1.97	1	FLOW	129.
				TIME	13.42
			2	FLOW	129.
				TIME	13.42
HYDROGRAPH AT	E3	1.51	1	FLOW	124.
				TIME	13.42
			2	FLOW	124.
				TIME	13.42
2 COMBINED AT	CP2	3.48	1	FLOW	254.
				TIME	13.42
			2	FLOW	254.
				TIME	13.42
ROUTED TO	DAM1	3.48	1	FLOW	89.
				TIME	16.17
			2	FLOW	254.
				TIME	13.42
** PEAK STAGES IN FEET **					
			1	STAGE	788.30
				TIME	16.17
			2	STAGE	.00
				TIME	.00
ROUTED TO	RCH3	3.48	1	FLOW	89.
				TIME	16.50
			2	FLOW	259.
				TIME	13.58
HYDROGRAPH AT	E4	.58	1	FLOW	50.
				TIME	12.67
			2	FLOW	50.
				TIME	12.67
1 COMBINED AT	CP3	4.06	1	FLOW	100.
				TIME	14.08
			2	FLOW	281.
				TIME	13.58

ROUTED TO	RCH3	4.06	1	FLOW	100.
				TIME	14.17
			2	FLOW	283.
				TIME	13.50

DROGRAPH AT	E5	.31	1	FLOW	32.
				TIME	12.58
			2	FLOW	32.
				TIME	12.58

COMBINED AT	CP4	4.37	1	FLOW	109.
				TIME	14.17
			2	FLOW	296.
				TIME	13.50

ROUTED TO	SIERRA	4.37	1	FLOW	109.
				TIME	14.25
			2	FLOW	231.
				TIME	14.33

** PEAK STAGES IN FEET **

1	STAGE	590.95
	TIME	14.25
2	STAGE	598.27
	TIME	14.33

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	129.46	805.00	.35	5.00	129.46	805.00	.35
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3842E+02 EXCESS= .0000E+00 OUTFLOW= .3734E+02 BASIN STORAGE= .1713E+01 PERCENT ERROR= -1.7

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	129.46	805.00	.35	5.00	129.46	805.00	.35
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3842E+02 EXCESS= .0000E+00 OUTFLOW= .3734E+02 BASIN STORAGE= .1713E+01 PERCENT ERROR= -1.7

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	89.48	990.00	.42	5.00	89.48	990.00	.42
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7945E+02 EXCESS= .0000E+00 OUTFLOW= .7773E+02 BASIN STORAGE= .1742E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	259.29	815.00	.41	5.00	259.29	815.00	.41
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7760E+02 EXCESS= .0000E+00 OUTFLOW= .7641E+02 BASIN STORAGE= .1490E+01 PERCENT ERROR= -.4

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	99.65	850.00	.40	5.00	99.65	850.00	.40
------	------	------	-------	--------	-----	------	-------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8822E+02 EXCESS= .0000E+00 OUTFLOW= .8720E+02 BASIN STORAGE= .1044E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	282.87	810.00	.40	5.00	282.87	810.00	.40
------	------	------	--------	--------	-----	------	--------	--------	-----

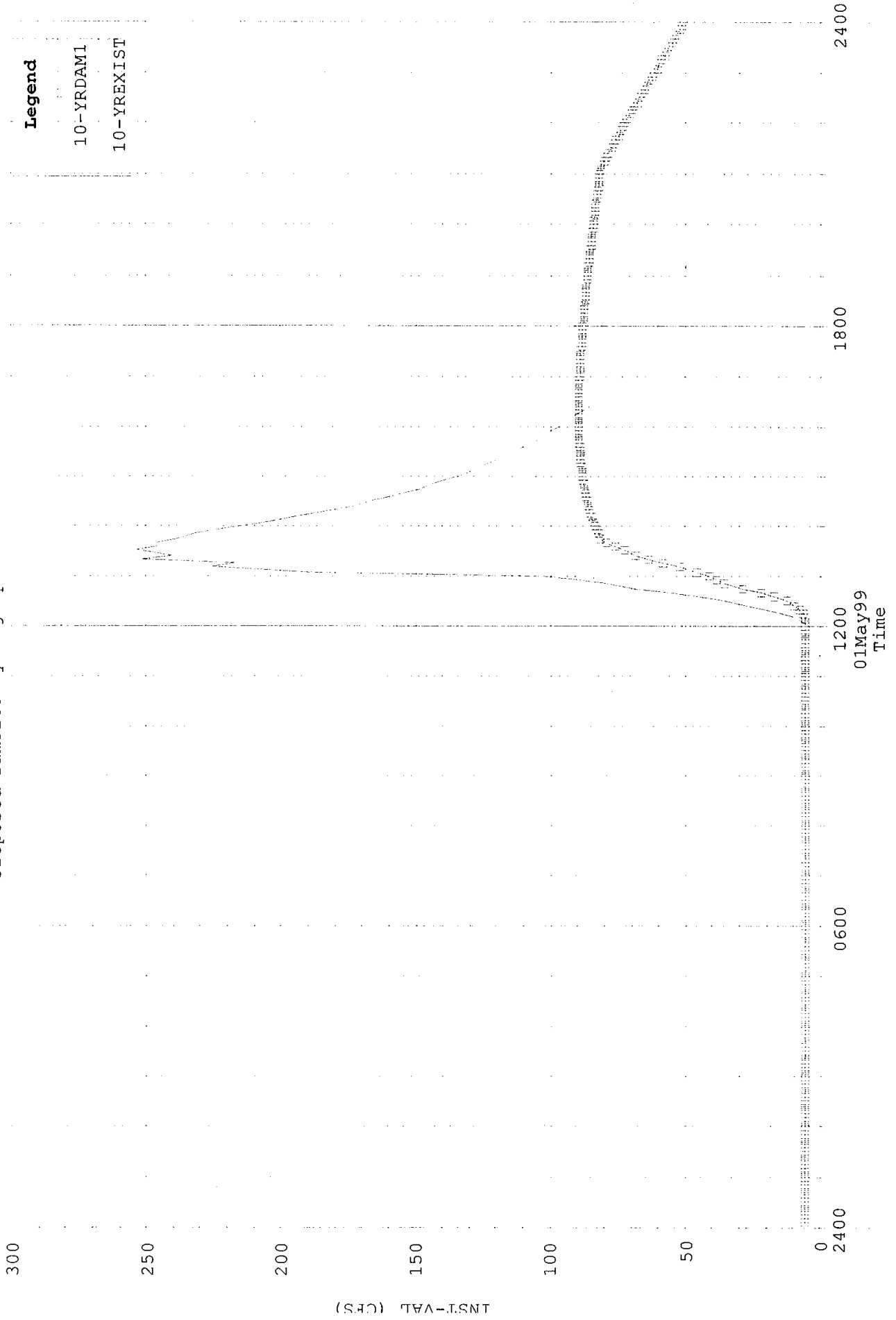
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8690E+02 EXCESS= .0000E+00 OUTFLOW= .8622E+02 BASIN STORAGE= .8883E+00 PERCENT ERROR= -.2

NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EVN10.DSS

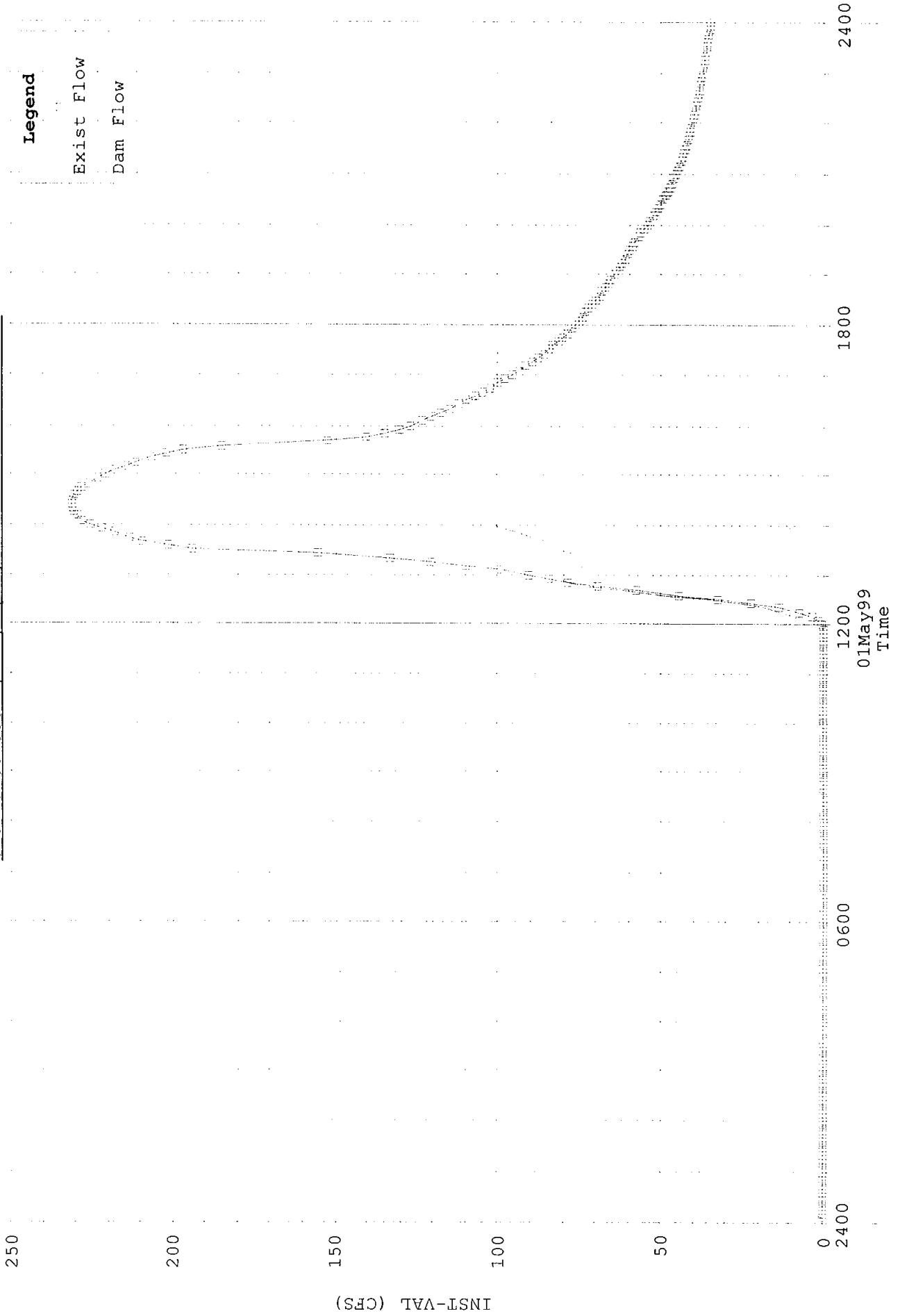
Pointer Utilization: .25
 Number of Records: 12
 File Size: 26.8 Kbytes
 Percent Inactive: .00

10-YR DAM1, EXIST FLOW
Proposed Damsite Hydrographs Evans Creek Block N



10-YR SIERRA FLOW

Sierra Street Hydrographs: Evans Creek Block N



25-YEAR, 24 HOUR

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID      PHASE II SUB-AREAS
2         ID
3         ID      EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID      25 YEAR 24 HOUR EVENT                DATE : MAY 1999
5         ID      NIMBUS ENGINEERS, RENO, NEVADA        FILE NAME: 2EVN25.DAT
6         ID
7         ID
8         ID      THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID      *****
10        ID      GONI CANYON                            Nimbus Job #: 9606      *
11        ID      Prepared by Nimbus Engineers          Date: August 1996   *
12        ID                                          File Name: GONI-5.DAT *
13        ID      *****
14        ID
15        ID      The 25-yr PH card rainfall depths were determined from the
16        ID      Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID      Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID      events and the methodology per the Draft Washoe County Hydrologic
19        ID      Criteria and Design Manual.
20        ID
21        ID      *****
22        ID      THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
23        ID      EXISTING CONDITIONS (PLAN 2)                                     *
24        ID      *****
25        IT      5 01MAY99    0005    288
26        IO      4          0
27        JP      2
28        *      DEPTH AREA REDUCTION FACTOR FROM NOAA
29        JR      PREC    0.99
30        *
31        KK      E1 SUBBASIN E1
32        BA      0.772
33        PH          0.31    0.56    0.93    1.14    1.30    1.63    2.02    2.40
34        LS          76
35        UD      0.79
36        ZW      A=25-YR C=FLOW F=E1FLOW
37        *
38        KK      E2 SUB-BASIN E2
39        BA      1.2
40        PH          0.31    0.56    0.93    1.09    1.22    1.48    1.84    2.20
41        LS          77
42        UD      0.99
43        ZW      A=25-YR C=FLOW F=E2FLOW
44        *
45        KK      CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
46        *      KO          2
47        HC      2
48        ZW      A=25-YR C=FLOW F=E1+E2
49        *

```

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10.....
44	KK	RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3									
45	RD	10800	0.026	0.07		TRAP	15	2			
	*										
46	KK	E3 SUBBASIN E3									
47	BA	1.51									
48	PH		0.31	0.56	0.93	1.09	1.22	1.48	1.87	2.26	
49	LS	81									
50	UD	1.24									
51	ZW	A=25-YR C=FLOW F=E3FLOW									
	*										
52	KK	CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3									
53	HC	2									
	*	ZW A=25-YR C=FLOW F=E1+E2+E3									
	*										
54	KK	DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS									
	* KO	2									
	* KP	1									
	* KM	PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.									
	* RS	1	STOR	0							
	* SA	.1354	.9137	1.954	3.63	3.71	3.79	3.87	3.95	5.12	6.
	* SA	7.97	8.92	9.93	10.18	10.45	10.94	11.55	12.67	14.15	15.
	* SE	765	775	785	795.89	796.33	796.76	797.2	797.64	803.16	808.
	* SE814.22	816.98	819.74	820.36	820.97	822.07	823.42	825.87	828.94	832	
	* SQ	0	0	0	0	13.46	38.09	69.97	107.73	114.35	120.
	* SQ126.56	129.43	132.24	218.16	401.01	881.31	1708.7	3654.6	6920.1	11031	
	* ZW	A=25-YR C=FLOW F=DAMFLOW									
	* ZW	C=STOR F=DAMSTOR									
	* ZW	C=STAGE F=DAMSTAGE									
	*										
55	KP	1									
56	KM	PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa									
57	RS	1	STOR	0							
58	SA	1.18	1.21	1.23	1.27	1.32	1.41	1.50	1.59	1.70	1.91
59	SA	2.83	3.91	3.97	4.72	6.68	9.07	10.37	11.09	12.75	15.70
60	SE	775.4	775.7	776.0	776.4	777.0	778.0	779.0	780.0	781.0	783.0
61	SE	790	796	796.3	800.4	808.69	816.98	820.36	822.10	825.87	832.0
62	SQ	5.4	12.6	21.4	35	40.8	49.9	57.5	64.3	70.4	81.3
63	SQ	92.1	95.43	112.2	134	145	155.4	244.5	907.5	3680.8	11060
64	ZW	A=25-YR C=FLOW F=30CMP									
65	ZW	C=STOR F=30CMP									
66	ZW	C=STAGE F=30CMP									
	*										
	* KO	2									
67	KP	2									
68	KM	EXISTING CONDITIONS (NO RESERVOIR)									
69	RN										
70	ZW	A=25-YR B=EXIST C=FLOW F=EXISTFLOW									
	*										

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
71	KK RCH3 ROUTE TO CP3
72	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
73	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
74	RD 4200 0.023 0.09 TRAP 20 2
	*
75	KK E4 SUBBASIN E4
76	BA 0.58
77	PH 0.31 0.56 0.93 1.08 1.19 1.43 1.76 2.09
78	LS 78
79	UD 0.57
80	ZW A=25-YR C=FLOW F=E4FLOW
	*
81	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
82	HC 2
83	ZW A=25-YR C=FLOW F=E4+CP2
	*
84	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
85	RD 3400 0.019 0.045 TRAP 20 2
	*
86	KK E5 SUB-BASIN E5
87	BA 0.31
88	PH 0.30 0.54 0.90 1.07 1.20 1.47 1.72 1.97
89	LS 80
90	UD 0.5
91	ZW A=25-YR C=FLOW F=E5FLOW
	*
92	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
93	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
94	HC 2
	* ZW A=25-YR C=FLOW F=SIERRA STREET
	*
95	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
96	KM STORAGE ROUTING AT SIERRA STREET
97	RS 1 STOR 0
98	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
99	SE 587 594 596 598 600 602 604
100	SQ 0 193 211 229 245 637 1000
101	ZW A=25-YR C=FLOW F=SIERRA STREET
	*
102	ZZ

SCHMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
29	E1	
	.	
35	.	E2
	.	.
41	CP1.....	
	V	
	V	
44	RCH1	
	.	
46	.	E3
	.	.
52	CP2.....	
	V	
	V	
54	DAM1	
	V	
	V	
71	RCH3	
	.	
75	.	E4
	.	.
81	CP3.....	
	V	
	V	
84	RCH3	
	.	
86	.	E5
	.	.
92	CP4.....	
	V	
	V	
95	SIERRA	

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 *
 FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 SEPTEMBER 1990 *
 VERSION 4.0 *
 *

 *
 U.S. ARMY CORPS OF ENGINEERS *
 HYDROLOGIC ENGINEERING CENTER *
 609 SECOND STREET *
 DAVIS, CALIFORNIA 95616 *
 (916) 756-1104 *
 *

RUN DATE 07/06/1999 TIME 13:45:50 *

PHASE II SUB-AREAS

EVANS CREEK BLOCK N HYDROLOGY
 25 YEAR 24 HOUR EVENT
 NIMBUS ENGINEERS, RENO, NEVADA

NIMBUS JOB # 9912
 DATE : MAY 1999
 FILE NAME: 2EVN25.DAT

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMBUS

 GONI CANYON
 Prepared by Nimbus Engineers
 Nimbus Job #: 9606 *
 Date: August 1996 *
 File Name: GONI-5.DAT *

The 25-yr PH card rainfall depths were determined from the
 Precipitation Frequency Study of the United States, NOAA Atlas 14,
 Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
 events and the methodology per the Draft Washoe County Hydrologic
 Criteria and Design Manual.

 THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND *
 EXISTING CONDITIONS (PLAN 2) *

26 IO OUTPUT CONTROL VARIABLES

IPRNT 4 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1MAY99 STARTING DATE
 ITIME 0005 STARTING TIME
 NQ 288 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2MAY99 ENDING DATE
 NDTIME 0000 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS

TOTAL TIME BASE 23.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
.99

29 KK * E1 * SUBBASIN E1

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.31 .56 .93 1.14 1.30 1.63 2.02 2.40 .00 .00 .00 .00

STORM AREA = .77

32 LS SCS LOSS RATE
STRTL .63 INITIAL ABSTRACTION
CRVNBR 76.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .79 LAG

UNIT HYDROGRAPH
49 END-OF-PERIOD ORDINATES

Table with 10 columns of ordinates: 14., 45., 86., 140., 212., 297., 368., 417., 444., 448., 443., 416., 385., 348., 303., 250., 205., 174., 147., 125., 108., 92., 79., 65., 56., 48., 41., 34., 29., 24., 21., 18., 15., 13., 11., 9., 8., 7., 6., 5., 4., 4., 3., 3., 2., 2., 1., 1., 0.

----DSS---ZOPEN: Version: 6-EA; Existing File Opened
Unit: 71, File: 2EVN25.DSS
---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/E1/FLOW/01MAY1999/5MIN/E1FLOW/

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

35 KK * E2 * SUB-BASIN E2

SUBBASIN RUNOFF DATA

36 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

37 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.31 .56 .93 1.09 1.22 1.48 1.84 2.20 .00 .00 .00 .00

STORM AREA = 1.20

38 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

39 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/E2/FLOW/01MAY1999/5MIN/E2FLOW/

** *** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

41 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1

42 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/
--DSS---ZWRITE Unit 71; Vers. 2: /25-YR/CP1/FLOW/01MAY1999/5MIN//

44 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
 * *

HYDROGRAPH ROUTING DATA

45 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 15.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * *
 46 KK * E3 * SUBBASIN E3
 * *

SUBBASIN RUNOFF DATA

47 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

48 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 .31 .56 .93 1.09 1.22 1.48 1.87 2.26 .00 .00 .00 .00

STORM AREA = 1.51

49 LS SCS LOSS RATE
 STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

50 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.24 LAG

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.

7. 6. 6. 5. 5. 4. 4. 3. 3. 3.

2. 2. 1. 1. 1. 0.

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/E3/FLOW/01MAY1999/5MIN/E3FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

*** **

52 KK *****
* *
* CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
* *

53 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

54 KK *****
* *
* DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
* *

*** **

5 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

7 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

8 SA AREA 1.2 1.2 1.2 1.3 1.3 1.4 1.5 1.6 1.7 1.9
2.8 3.9 4.0 4.7 6.7 9.1 10.4 11.1 12.8 15.7

60 SE ELEVATION 775.40 775.70 776.00 776.40 777.00 778.00 779.00 780.00 781.00 783.00
790.00 796.00 796.30 800.40 808.69 816.98 820.36 822.10 825.87 832.00

62 SQ DISCHARGE 5. 13. 21. 35. 41. 50. 58. 64. 70. 81.
92. 95. 112. 134. 145. 155. 245. 308. 3681. 11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE .00 .36 .72 1.22 2.00 3.37 4.82 6.37 8.01 11.62
ELEVATION 775.40 775.70 776.00 776.40 777.00 778.00 779.00 780.00 781.00 783.00

STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/DAM1/FLOW/01MAY1999/5MIN/30CMP/
 ----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/DAM1/STOR/01MAY1999/5MIN/30CMP/
 ----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/DAM1/STAGE/01MAY1999/5MIN/30CMP/

*** **

67 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

69 RN NO ROUTING

----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
 ----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
 ----DSS---ZWRITE Unit 71; Vers. 3: /25-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

 * *
 71 KK * RCH3 * ROUTE TO CP3
 * *

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

74 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	4200.	CHANNEL LENGTH
S	.0230	SLOPE
N	.090	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

 * *
 75 KK * E4 * SUBBASIN E4
 * *

SUBBASIN RUNOFF DATA

76 BA SUBBASIN CHARACTERISTICS
TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

77 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.31 .56 .93 1.08 1.19 1.43 1.76 2.09 .00 .00 .00 .00

STORM AREA = .58

78 LS SCS LOSS RATE
STRTL .56 INITIAL ABSTRACTION
CRVNBR 78.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

79 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .57 LAG

UNIT HYDROGRAPH
36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/E4/FLOW/01MAY1999/5MIN/E4FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

81 KK *****
* *
* CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
* *

92 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

84 KK *****
* *
* RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
* *

HYDROGRAPH ROUTING DATA

85 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	3400.	CHANNEL LENGTH
S	.0190	SLOPE
N	.045	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* **

 * *
 86 KK * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

87 BA SUBBASIN CHARACTERISTICS
 TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

88 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

.....	HYDRO-35	TP-40	TP-49
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.30	.54	.90	1.07	1.20	1.47	1.72	1.97	.00	.00	.00	.00

STORM AREA = .31

89 LS SCS LOSS RATE

STRTL	.50	INITIAL ABSTRACTION
CRVNBR	80.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

90 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.50	LAG
------	-----	-----

UNIT HYDROGRAPH
 32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

---DSS---ZWRITE Unit 71; Vers. 2: /25-YR/E5/FLOW/01MAY1999/5MIN/E5FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

 * * * * *
 92 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
 * * * * *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

94 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

 * * * * *
 95 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
 * * * * *

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

97 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

98 SA AREA .0 .5 .8 1.2 1.9 2.6 3.1

99 SE ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

00 SQ DISCHARGE 0. 193. 211. 229. 245. 637. 1000.

COMPUTED STORAGE-ELEVATION DATA

STORAGE .00 1.19 2.45 4.40 7.45 11.90 17.61
 ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

---DSS---ZWRITE Unit 71; Vers. 3: /25-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

*** **

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 3: /25-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	105.
				TIME	12.83
			2	FLOW	105.
				TIME	12.83
HYDROGRAPH AT	E2	1.20	1	FLOW	129.
				TIME	13.08
			2	FLOW	129.
				TIME	13.08
COMBINED AT	CP1	1.97	1	FLOW	231.
				TIME	13.00
			2	FLOW	231.
				TIME	13.00
ROUTED TO	RCH1	1.97	1	FLOW	236.
				TIME	13.42
			2	FLOW	236.
				TIME	13.42
HYDROGRAPH AT	E3	1.51	1	FLOW	204.
				TIME	13.33
			2	FLOW	204.
				TIME	13.33
COMBINED AT	CP2	3.48	1	FLOW	439.
				TIME	13.42
			2	FLOW	439.
				TIME	13.42
ROUTED TO	DAM1	3.48	1	FLOW	98.
				TIME	16.83
			2	FLOW	439.
				TIME	13.42
** PEAK STAGES IN FEET **					
ROUTED TO	RCH3	3.48	1	STAGE	796.05
				TIME	16.83
			2	STAGE	.00
				TIME	.00
HYDROGRAPH AT	E4	.58	1	FLOW	93.
				TIME	12.58
			2	FLOW	93.
				TIME	12.58
COMBINED AT	CP3	4.06	1	FLOW	113.
				TIME	13.58
			2	FLOW	471.
				TIME	13.50

ROUTED TO	RCH3	4.06	1	FLOW	114.
				TIME	13.75
			2	FLOW	480.
				TIME	13.67

HYDROGRAPH AT	E5	.31	1	FLOW	58.
				TIME	12.58
			2	FLOW	58.
				TIME	12.58

COMBINED AT	CP4	4.37	1	FLOW	150.
				TIME	12.67
			2	FLOW	496.
				TIME	13.67

ROUTED TO	SIERRA	4.37	1	FLOW	147.
				TIME	12.75
			2	FLOW	448.
				TIME	13.92

** PEAK STAGES IN FEET **

1	STAGE	592.32
	TIME	12.75
2	STAGE	601.04
	TIME	13.92

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
 (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
 COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00
 RCH1 MANE 5.00 236.14 805.00 .54 5.00 236.14 805.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5852E+02 EXCESS= .0000E+00 OUTFLOW= .5692E+02 BASIN STORAGE= .2221E+01 PERCENT ERROR= -1.1

FOR PLAN = 2 RATIO= .00
 RCH1 MANE 5.00 236.14 805.00 .54 5.00 236.14 805.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5852E+02 EXCESS= .0000E+00 OUTFLOW= .5692E+02 BASIN STORAGE= .2221E+01 PERCENT ERROR= -1.1

FOR PLAN = 1 RATIO= .00
 RCH3 MANE 5.00 98.30 1030.00 .48 5.00 98.30 1030.00 .48

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9262E+02 EXCESS= .0000E+00 OUTFLOW= .8988E+02 BASIN STORAGE= .2752E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00
 RCH3 MANE 5.00 440.44 815.00 .61 5.00 440.44 815.00 .61

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1154E+03 EXCESS= .0000E+00 OUTFLOW= .1140E+03 BASIN STORAGE= .1835E+01 PERCENT ERROR= -.4

FOR PLAN = 1 RATIO= .00
 RCH3 MANE 5.00 113.83 825.00 .48 5.00 113.83 825.00 .48

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1057E+03 EXCESS= .0000E+00 OUTFLOW= .1042E+03 BASIN STORAGE= .1561E+01 PERCENT ERROR= .0

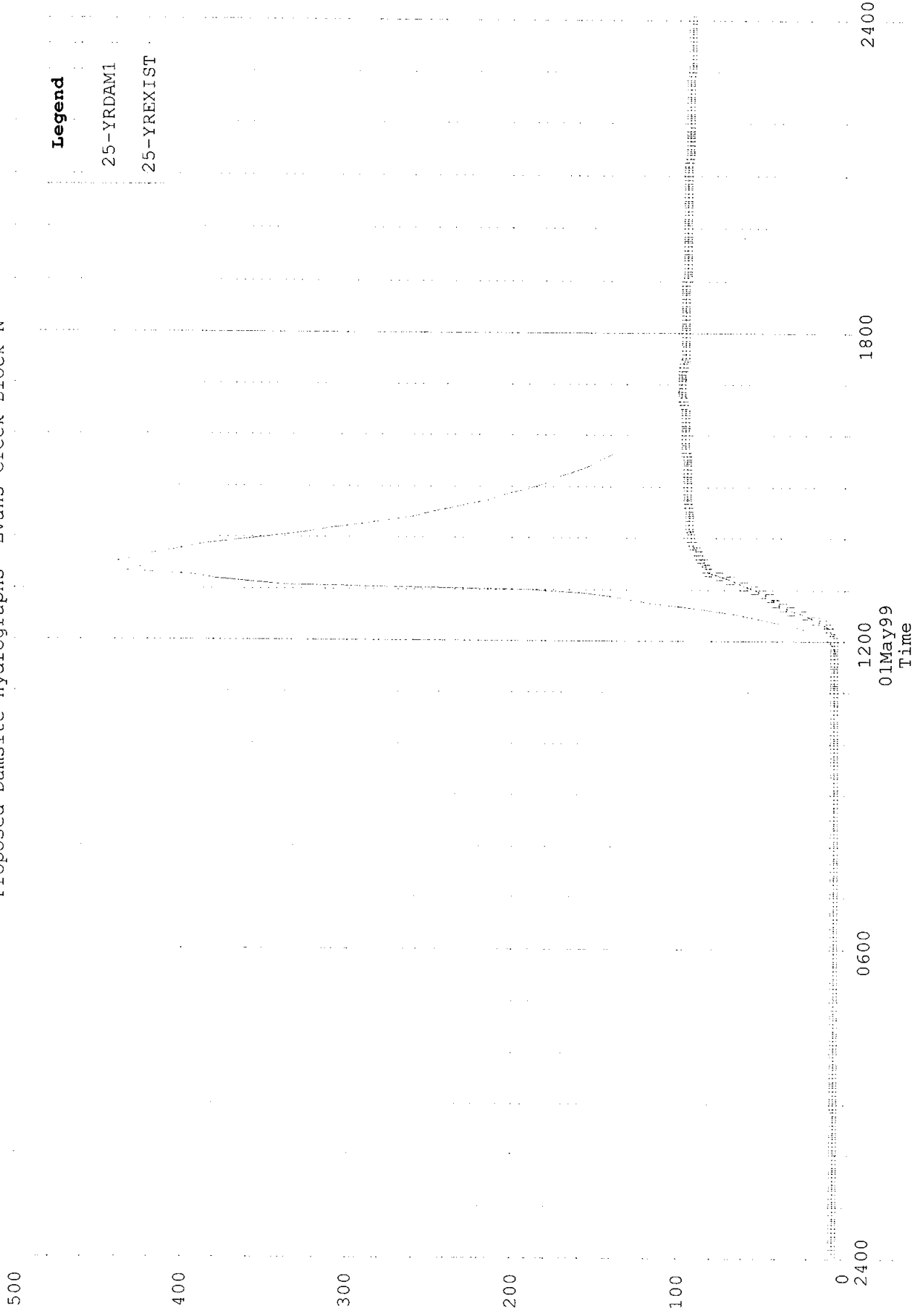
FOR PLAN = 2 RATIO= .00
 RCH3 MANE 5.00 480.30 820.00 .59 5.00 480.30 820.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1299E+03 EXCESS= .0000E+00 OUTFLOW= .1290E+03 BASIN STORAGE= .1092E+01 PERCENT ERROR= -.2

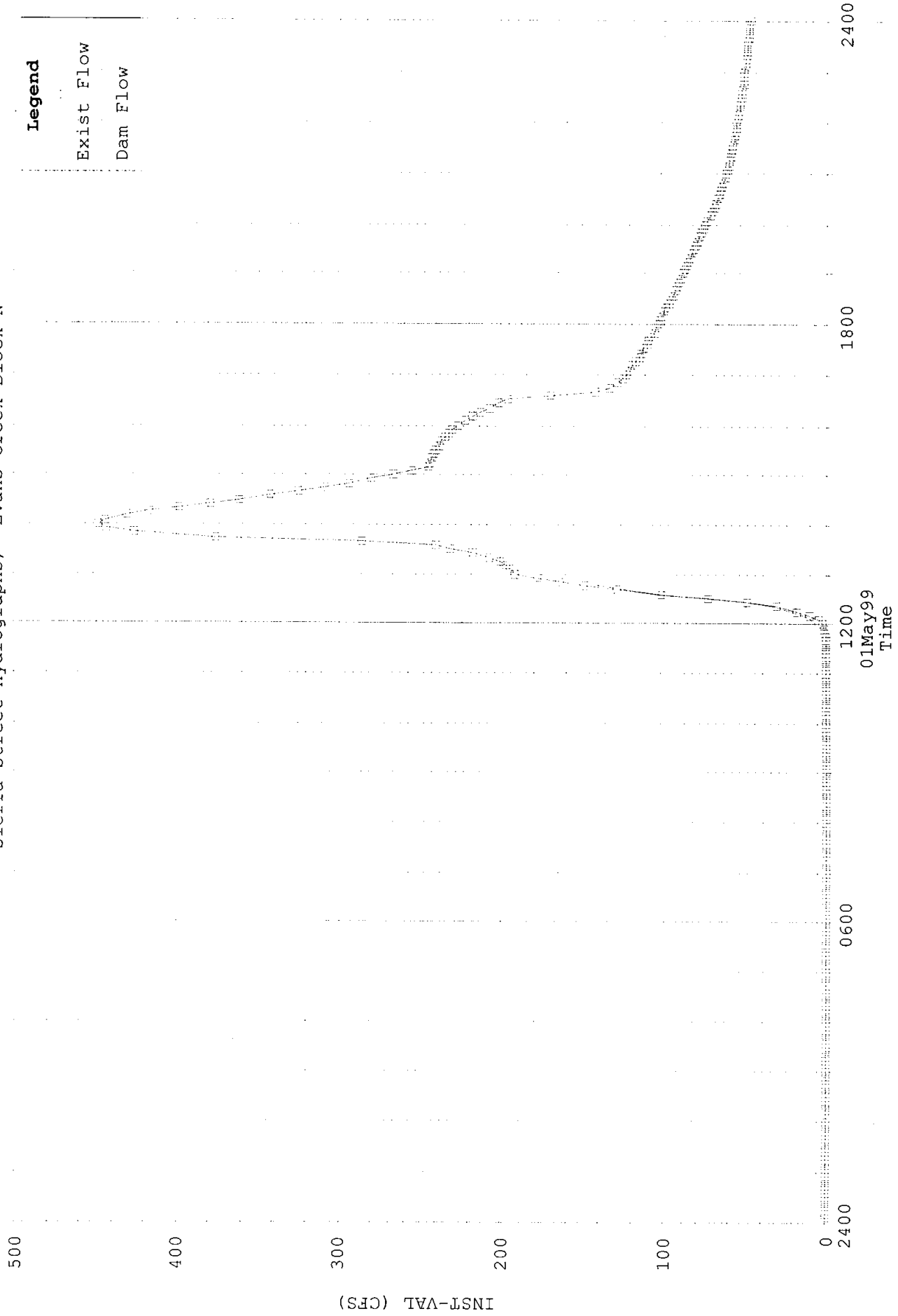
* NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EVN25.DSS
 Pointer Utilization: .25
 Number of Records: 17
 File Size: 42.3 Kbytes
 Percent Inactive: .00

25-YR DAM1, EXIST FLOW
Proposed Damsite Hydrographs Evans Creek Block N



25-YR SIERRA FLOW
Sierra Street Hydrographs; Evans Creek Block N



Legend

- Exist Flow
- Dam Flow

50-YEAR, 24 HOUR

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID    PHASE II SUB-AREAS
2         ID
3         ID    EVANS CREEK BLOCK N HYDROLOGY
4         ID    50 YEAR 24 HOUR EVENT
5         ID    NIMBUS ENGINEERS, RENO, NEVADA
6         ID
7         ID
8         ID    THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID    *****
10        ID    GONI CANYON
11        ID    Prepared by Nimbus Engineers
12        ID
13        ID    *****
14        ID
15        ID    The 50-yr PH card rainfall depths were determined from the
16        ID    Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID    Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID    events and the methodology per the Draft Washoe County Hydrologic
19        ID    Criteria and Design Manual.
20        ID
21        ID    *****
22        ID    THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND
23        ID    EXISTING CONDITIONS (PLAN 2)
24        ID    *****
25        IT    5 01MAY99 0005 288
26        IO    4 0
27        JP    2
28        *    DEPTH AREA REDUCTION FACTOR FROM NOAA
29        JR    PREC 0.99
30        *
31        KK    E1 SUBBASIN E1
32        BA    0.772
33        PH
34        LS    76
35        UD    0.79
36        *    ZW A=50-YR C=FLOW F=E1FLOW
37        *
38        KK    E2 SUB-BASIN E2
39        BA    1.2
40        PH
41        LS    77
42        UD    0.99
43        *    ZW A=50-YR C=FLOW F=E2FLOW
44        *
45        KK    CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
46        *    KO 2
47        HC    2
48        ZW    A=50-YR C=FLOW F=E1+E2
49        *

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

42 KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

43 RD 10800 0.026 0.07 TRAP 15 2

*

44 KK E3 SUBBASIN E3

45 BA 1.51

46 PH 0.38 0.70 1.16 1.31 1.43 1.67 2.10 2.53

47 LS 81

48 UD 1.24

* ZW A=50-YR C=FLOW F=E3FLOW

*

49 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3

50 HC 2

* ZW A=50-YR C=FLOW F=E1+E2+E3

*

51 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS

* KO 2

* KP 1

* KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.

* RS 1 STOR 0

* SA .1354 .9137 1.954 3.63 3.71 3.79 3.87 3.95 5.12 6.

* SA 7.97 8.92 9.93 10.18 10.45 10.94 11.55 12.67 14.15 15.

* SE 765 775 785 795.89 796.33 796.76 797.2 797.64 803.16 808.

* SE814.22 816.98 819.74 820.36 820.97 822.07 823.42 825.87 828.94 832

* SQ 0 0 0 0 13.46 38.09 69.97 107.73 114.35 120.

* SQ126.56 129.43 132.24 218.16 401.01 881.31 1708.7 3654.6 6920.1 11031

* ZW A=50-YR C=FLOW F=DAMFLOW

* ZW C=STOR F=DAMSTOR

* ZW C=STAGE F=DAMSTAGE

*

52 KP 1

53 KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

54 RS 1 STOR 0

55 SA 1.18 1.21 1.23 1.27 1.32 1.41 1.50 1.59 1.70 1.91

56 SA 2.83 3.91 3.97 4.72 6.68 9.07 10.37 11.09 12.75 15.70

57 SE 775.4 775.7 776.0 776.4 777.0 778.0 779.0 780.0 781.0 783.0

58 SE 790 796 796.3 800.4 808.69 816.98 820.36 822.10 825.87 832.0

59 SQ 5.4 12.6 21.4 35 40.8 49.9 57.5 64.3 70.4 81.3

60 SQ 92.1 95.43 112.2 134 145 155.4 244.5 307.5 3680.8 11060

61 ZW A=50-YR C=FLOW F=30CMP

62 ZW C=STOR F=30CMP

63 ZW C=STAGE F=30CMP

*

* KO 2

64 KP 2

65 KM EXISTING CONDITIONS (NO RESERVOIR)

66 RN

67 ZW A=50-YR B=EXIST C=FLOW F=EXISTFLOW

*

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
68	KK RCH3 ROUTE TO CP3
69	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
70	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
71	RD 4200 0.023 0.09 TRAP 20 2
	*
72	KK E4 SUBBASIN E4
73	BA 0.58
74	PH 0.38 0.70 1.16 1.29 1.40 1.61 1.98 2.35
75	LS 78
76	UD 0.57
	* ZW A=50-YR C=FLOW F=E4FLOW
	*
77	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
78	HC 2
79	ZW A=50-YR C=FLOW F=E4+CP2
	*
80	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
81	RD 3400 0.019 0.045 TRAP 20 2
	*
82	KK E5 SUB-BASIN E5
83	BA 0.31
84	PH 0.37 0.68 1.13 1.29 1.40 1.65 1.93 2.21
85	LS 80
86	UD 0.5
	* ZW A=50-YR C=FLOW F=E5FLOW
	*
87	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
88	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
89	HC 2
90	ZW A=50-YR C=FLOW F=SIERRA STREET
	*
91	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
92	KM STORAGE ROUTING AT SIERRA STREET
93	RS 1 STOR 0
94	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
95	SE 587 594 596 598 600 602 604
96	SQ 0 193 211 229 245 637 1000
97	ZW A=50-YR C=FLOW F=SIERRA STREET
	*
98	ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
 LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

29 E1
 .
 .
 34 . E2
 .
 .
 39 CP1.....
 V
 V
 42 RCH1
 .
 .
 44 . E3
 .
 .
 49 CP2.....
 V
 V
 51 DAM1
 V
 V
 68 RCH3
 .
 .
 72 . E4
 .
 .
 77 CP3.....
 V
 V
 80 RCH3
 .
 .
 82 . E5
 .
 .
 87 CP4.....
 V
 V
 91 SIERRA

**) RUNOFF ALSO COMPUTED AT THIS LOCATION

 *
 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * SEPTEMBER 1990 *
 * VERSION 4.0 *
 *
 * RUN DATE 06/29/1999 TIME 16:21:13 *
 *

 *
 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *
 *

PHASE II SUB-AREAS

EVANS CREEK BLOCK N HYDROLOGY NIMBUS JOB # 9912
 50 YEAR 24 HOUR EVENT DATE : MAY 1999
 NIMBUS ENGINEERS, RENO, NEVADA FILE NAME: 2EVN50.DAT

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMBUS

 GONI CANYON Nimbus Job #: 9606 *
 Prepared by Nimbus Engineers Date: August 1996 *
 File Name: GONI-5.DAT *

The 50-yr PH card rainfall depths were determined from the
 Precipitation Frequency Study of the United States, NOAA Atlas 14,
 Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
 events and the methodology per the Draft Washoe County Hydrologic
 Criteria and Design Manual.

 THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND *
 EXISTING CONDITIONS (PLAN 2) *

26 IO

OUTPUT CONTROL VARIABLES
 IPRNT 4 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1MAY99 STARTING DATE
 ITIME 0005 STARTING TIME
 NQ 288 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2MAY99 ENDING DATE
 NDTIME 0000 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS
 TOTAL TIME BASE 23.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP

MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 .99

 * *
 29 KK * E1 * SUBBASIN E1
 * *

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
 TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.38	.70	1.16	1.36	1.52	1.84	2.27	2.69	.00	.00	.00	.00

STORM AREA = .77

32 LS SCS LOSS RATE

STRTL	.63	INITIAL ABSTRACTION
CRVNBR	76.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .79 LAG

UNIT HYDROGRAPH
 49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

*** **

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

 * *
 34 KK * E2 * SUB-BASIN E2
 * *

SUBBASIN RUNOFF DATA

35 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

36 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.38 .70 1.16 1.31 1.43 1.67 2.07 2.47 .00 .00 .00 .00

STORM AREA = 1.20

37 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

38 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

* *
39 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* *

40 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened
Unit: 71, File: 2EVN50.DSS
--DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP1/FLOW/01MAY1999/5MIN//

* *
2 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

*

HYDROGRAPH ROUTING DATA

43 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 15.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

* *** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * *
 * E3 * SUBBASIN E3
 * *

44 KK

SUBBASIN RUNOFF DATA

45 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

46 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 .38 .70 1.16 1.31 1.43 1.67 2.10 2.53 .00 .00 .00 .00

STORM AREA = 1.51

47 LS SCS LOSS RATE
 STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

48 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.24 LAG

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.
7.	6.	6.	5.	5.	4.	4.	3.	3.	3.
2.	2.	1.	1.	1.	0.				

PRECIPITATION DATA

74 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.38	.70	1.16	1.29	1.40	1.61	1.98	2.35	.00	.00	.00	.00

STORM AREA = .58

75 LS SCS LOSS RATE

STRTL	.56	INITIAL ABSTRACTION
CRVNBR	78.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

76 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.57	LAG
------	-----	-----

UNIT HYDROGRAPH
36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

77 KK *****
* CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
* *

78 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

----DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
---DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

80 KK *****
* RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
* *

HYDROGRAPH ROUTING DATA

91 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	3400.	CHANNEL LENGTH
S	.0190	SLOPE
N	.045	CHANNEL ROUGHNESS COEFFICIENT

87 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH

* *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

89 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP4/FLOW/01MAY1999/5MIN/SIERRA STREET/

---DSS---ZWRITE Unit 71; Vers. 4: /50-YR/CP4/FLOW/01MAY1999/5MIN/EXISTFLOW/

91 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET

* *

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

93 RS STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

94 SA AREA .0 .5 .8 1.2 1.9 2.6 3.1

95 SE ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

96 SQ DISCHARGE 0. 193. 211. 229. 245. 637. 1000.

COMPUTED STORAGE-ELEVATION DATA

STORAGE .00 1.19 2.45 4.40 7.45 11.90 17.61
ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

---DSS---ZWRITE Unit 71; Vers. 4: /50-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 4: /50-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* **

 * *
 82 KK * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

83 BA SUBBASIN CHARACTERISTICS
 TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

84 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.37	.68	1.13	1.29	1.40	1.65	1.93	2.21	.00	.00	.00	.00

STORM AREA = .31

95 LS SCS LOSS RATE

STRTL	.50	INITIAL ABSTRACTION
CRVNBR	80.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

86 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.50	LAG
------	-----	-----

UNIT HYDROGRAPH
 32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

* **

 * *

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	154.
				TIME	12.83
			2	FLOW	154.
				TIME	12.83
HYDROGRAPH AT	E2	1.20	1	FLOW	190.
				TIME	13.08
			2	FLOW	190.
				TIME	13.08
COMBINED AT	CP1	1.97	1	FLOW	337.
				TIME	13.00
			2	FLOW	337.
				TIME	13.00
ROUTED TO	RCH1	1.97	1	FLOW	342.
				TIME	13.33
			2	FLOW	342.
				TIME	13.33
HYDROGRAPH AT	E3	1.51	1	FLOW	280.
				TIME	13.33
			2	FLOW	280.
				TIME	13.33
COMBINED AT	CP2	3.48	1	FLOW	622.
				TIME	13.33
			2	FLOW	622.
				TIME	13.33
ROUTED TO	DAM1	3.48	1	FLOW	132.
				TIME	16.00
			2	FLOW	622.
				TIME	13.33
** PEAK STAGES IN FEET **					
			1	STAGE	799.99
				TIME	16.00
			2	STAGE	.00
				TIME	.00
ROUTED TO	RCH3	3.48	1	FLOW	132.
				TIME	16.25
			2	FLOW	633.
				TIME	13.42
HYDROGRAPH AT	E4	.58	1	FLOW	139.
				TIME	12.58
			2	FLOW	139.
				TIME	12.58
COMBINED AT	CP3	4.06	1	FLOW	152.
				TIME	12.83
			2	FLOW	679.
				TIME	13.42

ROUTED TO	RCH3	4.06	1	FLOW	149.
				TIME	13.00
			2	FLOW	671.
				TIME	13.50

OROGRAPH AT	E5	.31	1	FLOW	85.
				TIME	12.50
			2	FLOW	85.
				TIME	12.50

2 COMBINED AT	CP4	4.37	1	FLOW	222.
				TIME	12.67
			2	FLOW	693.
				TIME	13.50

ROUTED TO	SIERRA	4.37	1	FLOW	200.
				TIME	12.92
			2	FLOW	652.
				TIME	13.75

** PEAK STAGES IN FEET **

1	STAGE	594.76
	TIME	12.92
2	STAGE	602.08
	TIME	13.75

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	INTERPOLATED TO COMPUTATION INTERVAL			VOLUME
						DT	PEAK	TIME TO PEAK	
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	342.46	800.00	.70	5.00	342.46	800.00	.70
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7550E+02 EXCESS= .0000E+00 OUTFLOW= .7373E+02 BASIN STORAGE= .2507E+01 PERCENT ERROR= -1.0

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	342.46	800.00	.70	5.00	342.46	800.00	.70
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7550E+02 EXCESS= .0000E+00 OUTFLOW= .7373E+02 BASIN STORAGE= .2507E+01 PERCENT ERROR= -1.0

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	131.80	975.00	.59	5.00	131.80	975.00	.59
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1123E+03 EXCESS= .0000E+00 OUTFLOW= .1095E+03 BASIN STORAGE= .2796E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	633.34	805.00	.78	5.00	633.34	805.00	.78
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1468E+03 EXCESS= .0000E+00 OUTFLOW= .1454E+03 BASIN STORAGE= .2048E+01 PERCENT ERROR= -.4

FOR PLAN = 1 RATIO= .00

RCH3	MANE	4.75	150.00	779.00	.59	5.00	149.19	780.00	.59
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1300E+03 EXCESS= .0000E+00 OUTFLOW= .1284E+03 BASIN STORAGE= .1597E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	671.42	810.00	.76	5.00	671.42	810.00	.76
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1659E+03 EXCESS= .0000E+00 OUTFLOW= .1651E+03 BASIN STORAGE= .1220E+01 PERCENT ERROR= -.3

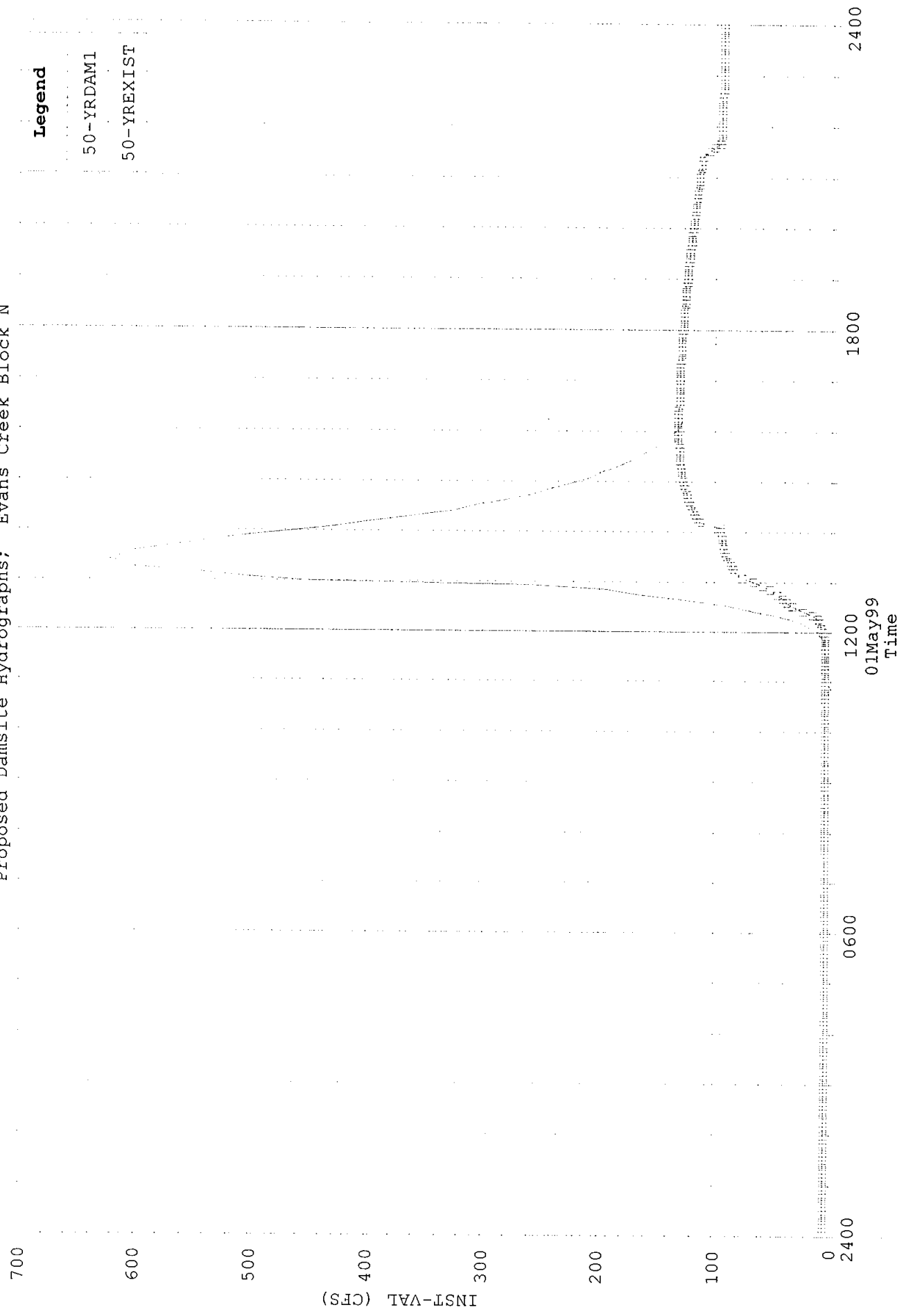
NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EVN50.DSS

Pointer Utilization: .25
Number of Records: 14
File Size: 29.5 Kbytes
Percent Inactive: .00

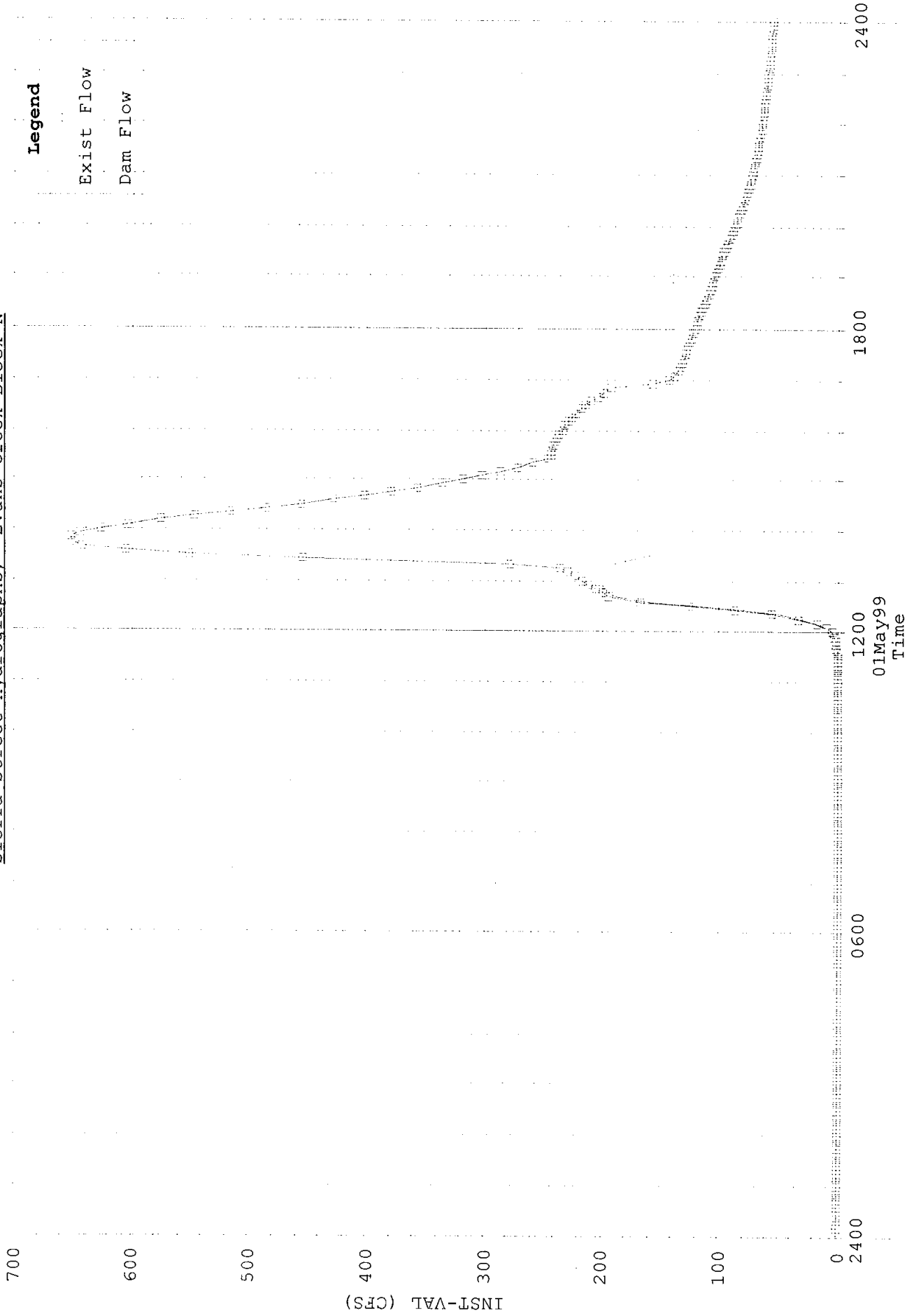
50-YR DAM1, EXIST FLOW
Proposed Damsite Hydrographs; Evans Creek Block N

- Legend**
50-YRDAMI
50-YREXIST



50-YR SIERRA FLOW

Sierra Street Hydrographs; Evans Creek Block N



100-YEAR, 24 HOUR

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID    PHASE II SUB-AREAS
2         ID
3         ID    EVANS CREEK BLOCK N HYDROLOGY
4         ID    100-YEAR 24 HOUR EVENT           NIMBUS JOB # 9912
5         ID    NIMBUS ENGINEERS, RENO, NEVADA    DATE : MAY 1999
6         ID                                           FILE NAME: 2EV100.DAT
7         ID
8         ID    THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID    *****
10        ID    GONI CANYON                       Nimbus Job #: 9606      *
11        ID    Prepared by Nimbus Engineers        Date: August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID    *****
14        ID
15        ID    The 100-yr PH card rainfall depths were determined from the
16        ID    Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID    Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID    events and the methodology per the Draft Washoe County Hydrologic
19        ID    Criteria and Design Manual.
20        ID
21        ID    *****
22        ID    THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
23        ID    EXISTING CONDITIONS (PLAN 2)                                     *
24        ID    *****
25        IT    5 01MAY99    0005    288
26        IO    4      0
27        JP    2
28        *    DEPTH AREA REDUCTION FACTOR FROM NOAA
29        JR    PREC    0.99
30        *
31        KK    E1 SUBBASIN E1
32        BA    0.772
33        PH           0.48    0.87    1.45    1.62    1.76    2.03    2.50    2.97
34        LS           76
35        UD    0.79
36        ZW    A=100-YR C=FLOW F=E1FLOW
37        *
38        KK    E2 SUB-BASIN E2
39        BA    1.2
40        PH           0.48    0.87    1.45    1.57    1.66    1.85    2.29    2.73
41        LS           77
42        UD    0.99
43        ZW    A=100-YR C=FLOW F=E2FLOW
44        *
45        KK    CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
46        * KO    2
47        HC    2
48        ZW    A=100-YR C=FLOW F=E1+E2
49        *

```

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44      KK      RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
45      RD      10800   0.026   0.07           TRAP      15      2
      *

46      KK      E3 SUBBASIN E3
47      BA      1.51
48      PH              0.48   0.87   1.45   1.57   1.66   1.85   2.33   2.80
49      LS              81
50      UD      1.24
51      ZW      A=100-YR C=FLOW F=E3FLOW
      *

52      KK      CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
53      HC      2
      * ZW A=100-YR C=FLOW F=E1+E2+E3
      *

54      KK      DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
      * KO      2
      * KP      1
      * KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.
      * RS      1      STOR      0
      * SA      .1354   .9137   1.954   3.63   3.71   3.79   3.87   3.95   5.12   6.
      * SA      7.97   8.92   9.93   10.18  10.45  10.94  11.55  12.67  14.15  15.
      * SE      765     775     785   795.89  796.33  796.76  797.2  797.64  803.16  808.
      * SE      814.22  816.98  819.74  820.36  820.97  822.07  823.42  825.87  828.94  832.
      * SQ      0       0       0       0      13.46  38.09  69.97  107.73  114.35  120.
      * SQ      126.56  129.43  132.24  218.16  401.01  881.31  1708.7  3654.6  6920.1  11031
      * ZW A=100-YR C=FLOW F=DAMFLOW
      * ZW C=STOR F=DAMSTOR
      * ZW C=STAGE F=DAMSTAGE
      *

55      KP      1
56      KM      PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa
57      RS      1      STOR      0
58      SA      1.18   1.21   1.23   1.27   1.32   1.41   1.50   1.59   1.70   1.91
59      SA      2.83   3.91   3.97   4.72   6.68   9.07   10.37  11.09  12.75  15.70
60      SE      775.4   775.7   776.0   776.4   777.0   778.0   779.0   780.0   781.0   783.0
61      SE      790     796     796.3   800.4   808.69  816.98  820.36  822.10  825.87  832.0
62      SQ      5.4    12.6   21.4   35     40.8   49.9   57.5   64.3   70.4   81.3
63      SQ      92.1   95.43  112.2   134    145    155.4   244.5   907.5  3680.8  11060
64      ZW      A=100-YR C=FLOW F=30CMP
65      ZW      C=STOR F=30CMP
66      ZW      C=STAGE F=30CMP
      *
      * KO      2

67      KP      2
68      KM      EXISTING CONDITIONS (NO RESERVOIR)
69      RN
70      ZW      A=100-YR B=EXIST C=FLOW F=EXISTFLOW
      *
    
```


LINE	ID	1	2	3	4	5	6	7	8	9	10
71	KK	RCH2 ROUTE TO CP3									
72	KM	ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3									
73	KM	HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES									
74	RD	4200	0.023	0.09		TRAP	20		2		
	*										
75	KK	E4 SUBBASIN E4									
76	BA	0.58									
77	PH			0.48	0.87	1.45	1.55	1.63	1.79	2.20	2.60
78	LS		78								
79	UD	0.57									
80	ZW	A=100-YR C=FLOW F=E4FLOW									
	*										
81	KK	CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3									
	* KO		2								
82	HC		2								
83	ZW	A=100-YR C=FLOW F=E4+CP2									
	*										
84	KK	RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S									
85	RD	3400	0.019	0.045		TRAP	20		2		
	*										
86	KK	E5 SUB-BASIN E5									
87	BA	0.31									
88	PH			0.47	0.85	1.41	1.54	1.63	1.83	2.14	2.44
89	LS		80								
90	UD	0.5									
91	ZW	A=100-YR C=FLOW F=E5FLOW									
	*										
92	KK	CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH									
	* KO		2								
93	KM	FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET									
94	HC		2								
	* ZW	A=100-YR C=FLOW F=SIERRA STREET									
	*										
95	KK	SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET									
96	KM	STORAGE ROUTING AT SIERRA STREET									
97	RS	1	STOR	0							
98	SA	0	0.51	0.76	1.2	1.88	2.59	3.13			
99	SE	587	594	596	598	600	602	604			
100	SQ	0	193	211	229	245	637	1000			
101	ZW	A=100-YR C=FLOW F=SIERRA STREET									
	*										
102	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK

PUT
 INE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

29	E1	
	.	
	.	
35	.	E2
	.	.
	.	.
41	CP1.....	
	V	
	V	
44	RCH1	
	.	
	.	
46	.	E3
	.	.
	.	.
52	CP2.....	
	V	
	V	
54	DAM1	
	V	
	V	
71	RCH2	
	.	
	.	
75	.	E4
	.	.
	.	.
81	CP3.....	
	V	
	V	
84	RCH3	
	.	
	.	
86	.	E5
	.	.
	.	.
92	CP4.....	
	V	
	V	
95	SIERRA	

*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                   *
*
* RUN DATE 07/06/1999 TIME 13:54:38 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS    *
* HYDROLOGIC ENGINEERING CENTER   *
*   609 SECOND STREET             *
*   DAVIS, CALIFORNIA 95616      *
*   (916) 756-1104               *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
100-YEAR 24 HOUR EVENT             DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA     FILE NAME: 2EV100.DAT

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THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
*****
GONI CANYON                        Nimbus Job #: 9606 *
Prepared by Nimbus Engineers        Date: August 1996 *
                                     File Name: GONI-5.DAT *
*****

```

The 100-yr PH card rainfall depths were determined from the Precipitation Frequency Study of the United States, NOAA Atlas 14, Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation events and the methodology per the Draft Washoe County Hydrologic Criteria and Design Manual.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND *
EXISTING CONDITIONS (PLAN 2) *
*****

```

26 IO

```

OUTPUT CONTROL VARIABLES
IPRNT      4  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE

```

IT

```

HYDROGRAPH TIME DATA
NMIN      5  MINUTES IN COMPUTATION INTERVAL
IDATE     1MAY99  STARTING DATE
ITIME     0005  STARTING TIME
NQ        288  NUMBER OF HYDROGRAPH ORDINATES
NDDATE    2MAY99  ENDING DATE
NDTIME    0000  ENDING TIME
ICENT     19  CENTURY MARK

```

```

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE     23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

JP

MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
.99

* *
29 KK * E1 * SUBBASIN E1
* *

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.48 .87 1.45 1.62 1.76 2.03 2.50 2.97 .00 .00 .00 .00
STORM AREA = .77

32 LS SCS LOSS RATE
STRTL .63 INITIAL ABSTRACTION
CRVNBR 76.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .79 LAG

UNIT HYDROGRAPH
49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened

Unit: 71, File: 2EV100.DSS

---DSS---ZWRITE Unit 71; Vers. 4: /100-YR/E1/FLOW/01MAY1999/5MIN/E1FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

* *
35 KK * E2 * SUB-BASIN E2
* *

SUBBASIN RUNOFF DATA

36 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

37 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.48 .87 1.45 1.57 1.66 1.85 2.29 2.73 .00 .00 .00 .00

STORM AREA = 1.20

38 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

39 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

---DSS---ZWRITE Unit 71; Vers. 4: /100-YR/E2/FLOW/01MAY1999/5MIN/E2FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

41 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1

42 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

----DSS---ZWRITE Unit 71; Vers. 5: /100-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/CP1/FLOW/01MAY1999/5MIN//

44 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
 * * *

HYDROGRAPH ROUTING DATA

45 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 15.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * * *
 46 KK * E3 * SUBBASIN E3
 * * *

SUBBASIN RUNOFF DATA

47 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

48 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 .48 .87 1.45 1.57 1.66 1.85 2.33 2.80 .00 .00 .00 .00

STORM AREA = 1.51

49 LS SCS LOSS RATE
 STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

50 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.24 LAG

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.

7. 6. 6. 5. 5. 4. 4. 3. 3. 3.
 2. 2. 1. 1. 1. 0.

---DSS---ZWRITE Unit 71; Vers. 4: /100-YR/E3/FLOW/01MAY1999/5MIN/E3FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

 * *
 52 KK * CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
 * *

53 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

 * *
 54 KK * DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 * *

55 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

57 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

58 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7

60 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

62 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.
		92.	95.	112.	134.	145.	155.	245.	308.	3681.	11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	.36	.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00

STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/DAM1/FLOW/01MAY1999/5MIN/30CMP/
 ----DSS---ZWRITE Unit 71; Vers. 5: /100-YR/DAM1/STOR/01MAY1999/5MIN/30CMP/
 ----DSS---ZWRITE Unit 71; Vers. 5: /100-YR/DAM1/STAGE/01MAY1999/5MIN/30CMP/

*** **

67 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

69 RN NO ROUTING

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
 ----DSS---ZWRITE Unit 71; Vers. 5: /100-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
 ----DSS---ZWRITE Unit 71; Vers. 5: /100-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

 * *
 71 KK * RCH2 * ROUTE TO CP3
 * *

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
 HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

74 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 4200. CHANNEL LENGTH
 S .0230 SLOPE
 N .090 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH2 ARE SAME AS FOR PLAN 1

 * *
 75 KK * E4 * SUBBASIN E4
 * *

SUBBASIN RUNOFF DATA

76 BA SUBBASIN CHARACTERISTICS
TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

77 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.48 .87 1.45 1.55 1.63 1.79 2.20 2.60 .00 .00 .00 .00

STORM AREA = .58

78 LS SCS LOSS RATE
STRTL .56 INITIAL ABSTRACTION
CRVNBR 78.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

79 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .57 LAG

UNIT HYDROGRAPH

36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

---DSS---ZWRITE Unit 71; Vers. 4: /100-YR/E4/FLOW/01MAY1999/5MIN/E4FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

.....

81 KK * CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3

82 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

.....

84 KK * RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S

HYDROGRAPH ROUTING DATA

85 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	3400.	CHANNEL LENGTH
S	.0190	SLOPE
N	.045	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

 * *
 86 KK * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

87 BA SUBBASIN CHARACTERISTICS
 TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

88 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

.....	HYDRO-35	TP-40	TP-49
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.47	.85	1.41	1.54	1.63	1.83	2.14	2.44	.00	.00	.00	.00

STORM AREA = .31

89 LS SCS LOSS RATE

STRTL	.50	INITIAL ABSTRACTION
CRVNBR	80.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

90 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .50 LAG

UNIT HYDROGRAPH
 32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

-----DSS---ZWRITE Unit 71; Vers. 4: /100-YR/ES/FLOW/01MAY1999/5MIN/ESFLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

* * * * *
92 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
* * * * *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

94 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

* * * * *
95 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
* * * * *

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

97 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

98 SA	AREA	.0	.5	.8	1.2	1.9	2.6	3.1
99 SE	ELEVATION	587.00	594.00	596.00	598.00	600.00	602.00	604.00
00 SQ	DISCHARGE	0.	193.	211.	229.	245.	637.	1000.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	1.19	2.45	4.40	7.45	11.90	17.61
ELEVATION	587.00	594.00	596.00	598.00	600.00	602.00	604.00

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

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PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 5: /100-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION		
				RATIO 1		
					.99	
HYDROGRAPH AT	E1	.77	1	FLOW	215.	
				TIME	12.83	
				2	FLOW	215.
				TIME	12.83	
HYDROGRAPH AT	E2	1.20	1	FLOW	264.	
				TIME	13.08	
				2	FLOW	264.
				TIME	13.08	
COMBINED AT	CP1	1.97	1	FLOW	469.	
				TIME	12.92	
				2	FLOW	469.
				TIME	12.92	
ROUTED TO	RCH1	1.97	1	FLOW	475.	
				TIME	13.25	
				2	FLOW	475.
				TIME	13.25	
HYDROGRAPH AT	E3	1.51	1	FLOW	372.	
				TIME	13.33	
				2	FLOW	372.
				TIME	13.33	
COMBINED AT	CP2	3.48	1	FLOW	847.	
				TIME	13.25	
				2	FLOW	847.
				TIME	13.25	
ROUTED TO	DAM1	3.48	1	FLOW	139.	
				TIME	15.75	
				2	FLOW	847.
				TIME	13.25	
				** PEAK STAGES IN FEET **		
			1	STAGE	804.01	
				TIME	15.83	
			2	STAGE	.00	
				TIME	.00	
ROUTED TO	RCH2	3.48	1	FLOW	139.	
				TIME	16.00	
				2	FLOW	857.
				TIME	13.33	
HYDROGRAPH AT	E4	.58	1	FLOW	196.	
				TIME	12.58	
				2	FLOW	196.
				TIME	12.58	
COMBINED AT	CP3	4.06	1	FLOW	210.	
				TIME	12.75	
				2	FLOW	921.
				TIME	13.33	

ROUTED TO	RCH3	4.06	1	FLOW	207.
				TIME	12.92
			2	FLOW	915.
				TIME	13.42

DROGRAPH AT	E5	.31	1	FLOW	119.
				TIME	12.50
			2	FLOW	119.
				TIME	12.50

COMBINED AT	CP4	4.37	1	FLOW	310.
				TIME	12.67
			2	FLOW	941.
				TIME	13.42

ROUTED TO	SIERRA	4.37	1	FLOW	231.
				TIME	13.08
			2	FLOW	880.
				TIME	13.67

** PEAK STAGES IN FEET **

1	STAGE	598.20
	TIME	13.08
2	STAGE	603.34
	TIME	13.67

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	475.31	795.00	.87	5.00	475.31	795.00	.87
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .9298E+02 EXCESS= .0000E+00 OUTFLOW= .9108E+02 BASIN STORAGE= .2794E+01 PERCENT ERROR= -1.0

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	475.31	795.00	.87	5.00	475.31	795.00	.87
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9298E+02 EXCESS= .0000E+00 OUTFLOW= .9108E+02 BASIN STORAGE= .2794E+01 PERCENT ERROR= -1.0

FOR PLAN = 1 RATIO= .00

RCH2	MANE	5.00	138.78	965.00	.67	5.00	138.78	965.00	.67
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1288E+03 EXCESS= .0000E+00 OUTFLOW= .1255E+03 BASIN STORAGE= .3394E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH2	MANE	5.00	856.92	800.00	.96	5.00	856.92	800.00	.96
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1794E+03 EXCESS= .0000E+00 OUTFLOW= .1778E+03 BASIN STORAGE= .2258E+01 PERCENT ERROR= -.4

FOR PLAN = 1 RATIO= .00

RCH3	MANE	4.25	208.55	773.50	.68	5.00	207.48	775.00	.69
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1503E+03 EXCESS= .0000E+00 OUTFLOW= .1484E+03 BASIN STORAGE= .1943E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	914.76	805.00	.93	5.00	914.76	805.00	.93
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2032E+03 EXCESS= .0000E+00 OUTFLOW= .2023E+03 BASIN STORAGE= .1344E+01 PERCENT ERROR= -.2

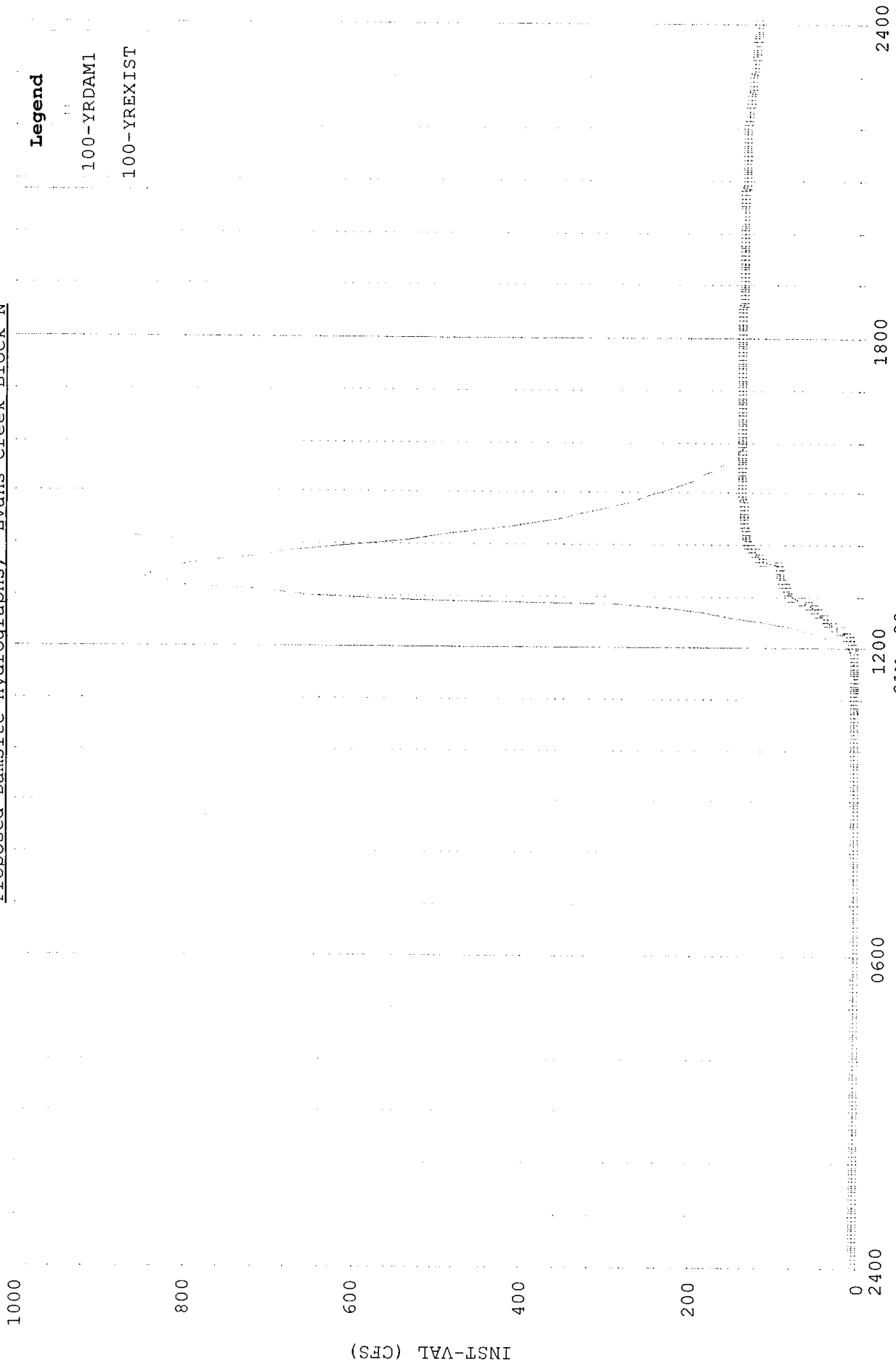
* NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EV100.DSS

Pointer Utilization: .25
Number of Records: 17
File Size: 41.8 Kbytes
Percent Inactive: .00

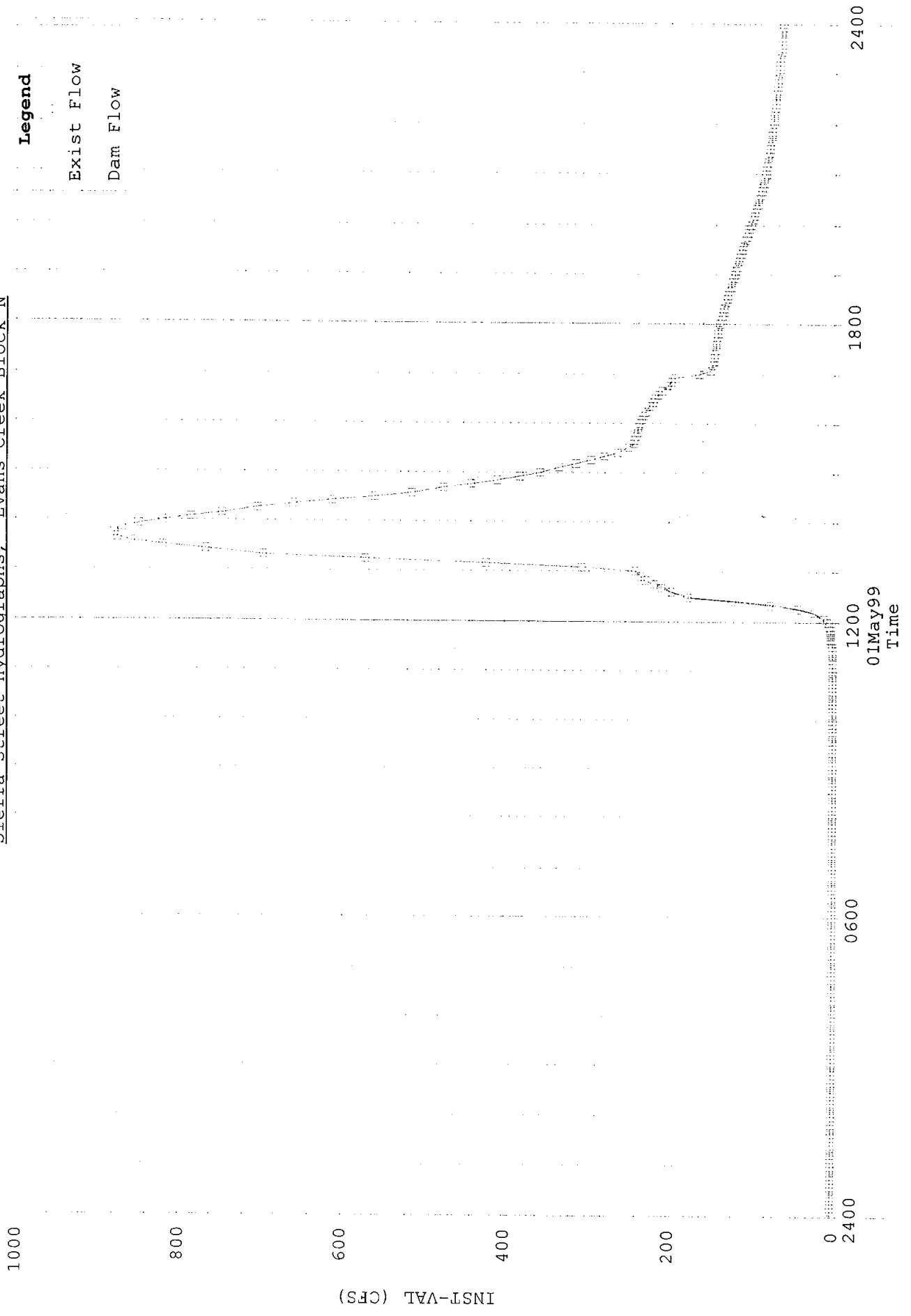
100-YR DAM1, EXIST FLOW
Proposed Damsite Hydrographs; Evans Creek Block N

- Legend**
100-YRDAMI
100-YREXIST



100-YR SIERRA FLOW

Sierra Street Hydrographs; Evans Creek Block N



Legend

Exist Flow

Dam Flow

500-YEAR, 24 HOUR

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*DIAGRAM
1         ID    PHASE II SUB-AREAS
2         ID
3         ID    EVANS CREEK BLOCK N HYDROLOGY           NIMBUS JOB # 9912
4         ID    500 YEAR 24 HOUR EVENT                 DATE : MAY 1999
5         ID    NIMBUS ENGINEERS, RENO, NEVADA          FILE NAME: 2EV500.DAT
6         ID
7         ID
8         ID    THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID    *****
10        ID    GONI CANYON                               Nimbus Job #: 9606      *
11        ID    Prepared by Nimbus Engineers             Date: August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID    *****
14        ID
15        ID    The 500-yr PH card rainfall depths were determined from the
16        ID    Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID    Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID    events and the methodology per the Draft Washoe County Hydrologic
19        ID    Criteria and Design Manual.
20        ID
21        ID    *****
22        ID    THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
23        ID    EXISTING CONDITIONS (PLAN 2)                                     *
24        ID    *****
*
25        IT    5 01MAY99    0005    288
26        IO    4      0
27        JP    2
* DEPTH AREA REDUCTION FACTOR FROM NOAA
28        JR    PREC    0.99
*
29        KK    E1 SUBBASIN E1
30        BA    0.772
31        PH           0.59    1.07    1.79    2.00    2.16    2.50    3.08    3.66
32        LS           76
33        UD    0.79
34        ZW    A=500-YR C=FLOW F=E1FLOW
*
35        KK    E2 SUB-BASIN E2
36        BA    1.2
37        PH           0.59    1.07    1.79    1.94    2.05    2.28    2.82    3.36
38        LS           77
39        UD    0.99
40        ZW    A=500-YR C=FLOW F=E2FLOW
*
41        KK    CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* KO           2
42        HC           2
43        ZW    A=500-YR C=FLOW F=E1+E2
*

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44 KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

45 RD 10800 0.026 0.07 TRAP 15 2

*

46 KK E3 SUBBASIN E3

47 BA 1.51

48 PH 0.59 1.07 1.79 1.94 2.05 2.28 2.86 3.44

49 LS 81

50 UD 1.24

51 ZW A=500-YR C=FLOW F=E3FLOW

*

52 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3

53 HC 2

* ZW A=500-YR C=FLOW F=E1+E2+E3

*

54 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS

* KO 2

* KP 1

* KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.

* RS 1 STOR 0

* SA .1354 .9137 1.954 3.63 3.71 3.79 3.87 3.95 5.12 6.

* SA 7.97 8.92 9.93 10.18 10.45 10.94 11.55 12.67 14.15 15.

* SE 765 775 785 795.89 796.33 796.76 797.2 797.64 803.16 808.

* SE814.22 816.98 819.74 820.36 820.97 822.07 823.42 825.87 828.94 832

* SQ 0 0 0 0 13.46 38.09 69.97 107.73 114.35 120.

* SQ126.56 129.43 132.24 218.16 401.01 881.31 1708.7 3654.6 6920.1 11031

* ZW A=500-YR C=FLOW F=DAMFLOW

* ZW C=STOR F=DAMSTOR

* ZW C=STAGE F=DAMSTAGE

*

55 KP 1

56 KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

57 RS 1 STOR 0

58 SA 1.18 1.21 1.23 1.27 1.32 1.41 1.50 1.59 1.70 1.91

59 SA 2.83 3.91 3.97 4.72 6.68 9.07 10.37 11.09 12.75 15.70

60 SE 775.4 775.7 776.0 776.4 777.0 778.0 779.0 780.0 781.0 783.0

61 SE 790 796 796.3 800.4 808.69 816.98 820.36 822.10 825.87 832.0

62 SQ 5.4 12.6 21.4 35 40.8 49.9 57.5 64.3 70.4 81.3

63 SQ 92.1 95.43 112.2 134 145 155.4 244.5 907.5 3680.8 11060

64 ZW A=500-YR C=FLOW F=30CMP

65 ZW C=STOR F=30CMP

66 ZW C=STAGE F=30CMP

*

* KO 2

67 KP 2

68 KM EXISTING CONDITIONS (NO RESERVOIR)

69 RN

70 ZW A=500-YR B=EXIST C=FLOW F=EXISTFLOW

*

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
71	KK RCH3 ROUTE TO CP3
72	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
73	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
74	RD 4200 0.023 0.09 TRAP 20 2
	*
75	KK E4 SUBBASIN E4
76	BA 0.58
77	PH 0.59 1.07 1.79 1.91 2.01 2.20 2.70 3.19
78	LS 78
79	UD 0.57
80	ZW A=500-YR C=FLOW F=E4FLOW
	*
81	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
82	HC 2
83	ZW A=500-YR C=FLOW F=E4+CP2
	*
84	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
85	RD 3400 0.019 0.045 TRAP 20 2
	*
86	KK E5 SUB-BASIN E5
87	BA 0.31
88	PH 0.58 1.05 1.75 1.90 2.01 2.25 2.63 3.00
89	LS 80
90	UD 0.5
91	ZW A=500-YR C=FLOW F=E5FLOW
	*
92	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
93	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
94	HC 2
	* ZW A=500-YR C=FLOW F=SIERRA STREET
95	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
96	KM STORAGE ROUTING AT SIERRA STREET
97	RS 1 STOR 0
98	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
99	SE 587 594 596 598 600 602 604
100	SQ 0 193 211 229 245 637 2050
101	ZW A=500-YR C=FLOW F=SIERRA STREET
	*
102	ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
29	E1	
	.	
35	.	E2
	.	.
41	CP1.....	
	V	
	V	
44	RCH1	
	.	
46	.	E3
	.	.
52	CP2.....	
	V	
	V	
54	DAM1	
	V	
	V	
71	RCH3	
	.	
75	.	E4
	.	.
81	CP3.....	
	V	
	V	
84	RCH3	
	.	
86	.	E5
	.	.
92	CP4.....	
	V	
	V	
95	SIERRA	

*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990             *
*   VERSION 4.0               *
*
* RUN DATE 07/06/1999 TIME 14:03:11 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET          *
* DAVIS, CALIFORNIA 95616     *
*   (916) 756-1104            *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
500 YEAR 24 HOUR EVENT             DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA     FILE NAME: 2EV500.DAT

```

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMBUS

```

*****
GONI CANYON                        Nimbus Job #: 9606      *
Prepared by Nimbus Engineers       Date: August 1996   *
                                   File Name: GONI-5.DAT  *
*****

```

The 500-yr PH card rainfall depths were determined from the Precipitation Frequency Study of the United States, NOAA Atlas 14, Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation events and the methodology per the Draft Washoe County Hydrologic Criteria and Design Manual.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND *
EXISTING CONDITIONS (PLAN 2)                               *
*****

```

26 IO OUTPUT CONTROL VARIABLES

```

IPRNT      4 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL     0. HYDROGRAPH PLOT SCALE

```

IT HYDROGRAPH TIME DATA

```

NMIN      5 MINUTES IN COMPUTATION INTERVAL
IDATE     1MAY99 STARTING DATE
ITIME     0005 STARTING TIME
NQ        288 NUMBER OF HYDROGRAPH ORDINATES
NDDATE    2MAY99 ENDING DATE
NDTIME    0000 ENDING TIME
ICENT     19 CENTURY MARK

```

```

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE     23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

JP MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 .99

 * *
 29 KK * E1 * SUBBASIN E1
 * *

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
 TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.59	1.07	1.79	2.00	2.16	2.50	3.08	3.66	.00	.00	.00	.00

STORM AREA = .77

32 LS SCS LOSS RATE

STRTL	.63	INITIAL ABSTRACTION
CRVNBR	76.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .79 LAG

UNIT HYDROGRAPH
 49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened
 Unit: 71, File: 2EV500.DSS

---DSS---ZWRITE Unit 71; Vers. 3: /500-YR/E1/FLOW/01MAY1999/5MIN/E1FLOW/

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

 * *
 35 KK * E2 * SUB-BASIN E2
 * *

SUBBASIN RUNOFF DATA

36 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

37 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.59 1.07 1.79 1.94 2.05 2.28 2.82 3.36 .00 .00 .00 .00

STORM AREA = 1.20

38 LS SCS LOSS RATE
STRTL .60 INITIAL ABSTRACTION
CRVNBR 77.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

39 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

---DSS---ZWRITE Unit 71; Vers. 3: /500-YR/E2/FLOW/01MAY1999/5MIN/E2FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

* *
41 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* *

42 HC HYDROGRAPH COMBINATION
ICOMB 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 3: /500-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/
--DSS---ZWRITE Unit 71; Vers. 3: /500-YR/CP1/FLOW/01MAY1999/5MIN//

44 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
 * * *

HYDROGRAPH ROUTING DATA

45 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 15.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * *
 46 KK * E3 * SUBBASIN E3
 * *

SUBBASIN RUNOFF DATA

17 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

48 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 .59 1.07 1.79 1.94 2.05 2.28 2.86 3.44 .00 .00 .00 .00

STORM AREA = 1.51

9 LS SCS LOSS RATE
 STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

50 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.24 LAG

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.

7. 6. 6. 5. 5. 4. 4. 3. 3. 3.
 2. 2. 1. 1. 1. 0.

---DSS---ZWRITE Unit 71; Vers. 3: /500-YR/E3/FLOW/01MAY1999/5MIN/E3FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

 * *
 52 KK * CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
 * *

53 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

 * *
 54 KK * DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 * *

*** **

55 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

57 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

58 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7

60 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

62 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.
		92.	95.	112.	134.	145.	155.	245.	908.	3681.	11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	.36	.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00

STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.97	832.00

```

---DSS---ZWRITE Unit 71; Vers. 4: /500-YR/DAM1/FLOW/01MAY1999/5MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 4: /500-YR/DAM1/STOR/01MAY1999/5MIN/30CMP/
-----DSS---ZWRITE Unit 71; Vers. 4: /500-YR/DAM1/STAGE/01MAY1999/5MIN/30CMP/

```

*** **

67 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

69 RN NO ROUTING

```

---DSS---ZWRITE Unit 71; Vers. 4: /500-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 4: /500-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
-----DSS---ZWRITE Unit 71; Vers. 4: /500-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

```

```

*****
*
71 KK * RCH3 * ROUTE TO CP3
*
*****

```

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

74 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	4200.	CHANNEL LENGTH
S	.0230	SLOPE
N	.090	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

```

*****
*
75 KK * E4 * SUBBASIN E4
*
*****

```

SUBBASIN RUNOFF DATA

76 BA SUBBASIN CHARACTERISTICS
TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

77 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.59 1.07 1.79 1.91 2.01 2.20 2.70 3.19 .00 .00 .00 .00
STORM AREA = .58

78 LS SCS LOSS RATE
STRTL .56 INITIAL ABSTRACTION
CRVNBR 78.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

79 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .57 LAG

UNIT HYDROGRAPH
36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

--DSS--ZWRITE Unit 71; Vers. 3: /500-YR/E4/FLOW/01MAY1999/5MIN/E4FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

81 KK * CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3

82 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

--DSS--ZWRITE Unit 71; Vers. 3: /500-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
--DSS--ZWRITE Unit 71; Vers. 3: /500-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

84 KK * RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S

HYDROGRAPH ROUTING DATA

85 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 3400. CHANNEL LENGTH
 S .0190 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

* **

86 KK

* *
 * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

87 BA

SUBBASIN CHARACTERISTICS

TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

88 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.58	1.05	1.75	1.90	2.01	2.25	2.63	3.00	.00	.00	.00	.00

STORM AREA = .31

89 LS

SCS LOSS RATE

STRTL .50 INITIAL ABSTRACTION
 CRVNBR 80.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

90 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .50 LAG

UNIT HYDROGRAPH

32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

----DSS---ZWRITE Unit 71; Vers. 3: /500-YR/E5/FLOW/01MAY1999/5MIN/E5FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

* *
92 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
* *

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

94 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

* *
95 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
* *

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

97 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

98 SA AREA .0 .5 .8 1.2 1.9 2.6 3.1

99 SE ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

00 SQ DISCHARGE 0. 193. 211. 229. 245. 637. 2050.

COMPUTED STORAGE-ELEVATION DATA

STORAGE .00 1.19 2.45 4.40 7.45 11.90 17.61
ELEVATION 587.00 594.00 596.00 598.00 600.00 602.00 604.00

---DSS---ZWRITE Unit 71; Vers. 4: /500-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

*** **

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

---DSS---ZWRITE Unit 71; Vers. 4: /500-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	334.
				TIME	12.83
			2	FLOW	334.
				TIME	12.83
HYDROGRAPH AT	E2	1.20	1	FLOW	413.
				TIME	13.00
			2	FLOW	413.
				TIME	13.00
COMBINED AT	CP1	1.97	1	FLOW	732.
				TIME	12.92
			2	FLOW	732.
				TIME	12.92
ROUTED TO	RCH1	1.97	1	FLOW	731.
				TIME	13.25
			2	FLOW	731.
				TIME	13.25
HYDROGRAPH AT	E3	1.51	1	FLOW	546.
				TIME	13.25
			2	FLOW	546.
				TIME	13.25
COMBINED AT	CP2	3.48	1	FLOW	1278.
				TIME	13.25
			2	FLOW	1278.
				TIME	13.25
ROUTED TO	DAM1	3.48	1	FLOW	151.
				TIME	17.92
			2	FLOW	1278.
				TIME	13.25
** PEAK STAGES IN FEET **					
ROUTED TO	RCH3	3.48	1	STAGE	813.13
				TIME	18.00
			2	STAGE	.00
				TIME	.00
HYDROGRAPH AT	E4	.58	1	FLOW	304.
				TIME	12.58
			2	FLOW	304.
				TIME	12.58
COMBINED AT	CP3	4.06	1	FLOW	340.
				TIME	12.67
			2	FLOW	1375.
				TIME	13.33

ROUTED TO	RCH3	4.06	1	FLOW	340.
				TIME	12.75
			2	FLOW	1371.
				TIME	13.33

DROGRAPH AT	E5	.31	1	FLOW	183.
				TIME	12.50
			2	FLOW	183.
				TIME	12.50

COMBINED AT	CP4	4.37	1	FLOW	500.
				TIME	12.67
			2	FLOW	1415.
				TIME	13.33

ROUTED TO	SIERRA	4.37	1	FLOW	418.
				TIME	12.92
			2	FLOW	1409.
				TIME	13.42

** PEAK STAGES IN FEET **

1	STAGE	600.88
	TIME	12.92
2	STAGE	603.09
	TIME	13.42

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	731.38	795.00	1.30	5.00	731.38	795.00	1.30
------	------	------	--------	--------	------	------	--------	--------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1394E+03 EXCESS= .0000E+00 OUTFLOW= .1372E+03 BASIN STORAGE= .3437E+01 PERCENT ERROR= -.9

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	731.38	795.00	1.30	5.00	731.38	795.00	1.30
------	------	------	--------	--------	------	------	--------	--------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1394E+03 EXCESS= .0000E+00 OUTFLOW= .1372E+03 BASIN STORAGE= .3437E+01 PERCENT ERROR= -.9

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	150.57	1095.00	.75	5.00	150.57	1095.00	.75
------	------	------	--------	---------	-----	------	--------	---------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1438E+03 EXCESS= .0000E+00 OUTFLOW= .1399E+03 BASIN STORAGE= .3926E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	1281.49	800.00	1.41	5.00	1281.49	800.00	1.41
------	------	------	---------	--------	------	------	---------	--------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2639E+03 EXCESS= .0000E+00 OUTFLOW= .2622E+03 BASIN STORAGE= .2743E+01 PERCENT ERROR= -.4

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	340.34	765.00	.81	5.00	340.34	765.00	.81
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1777E+03 EXCESS= .0000E+00 OUTFLOW= .1755E+03 BASIN STORAGE= .2237E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	4.70	1371.34	799.79	1.38	5.00	1371.10	800.00	1.38
------	------	------	---------	--------	------	------	---------	--------	------

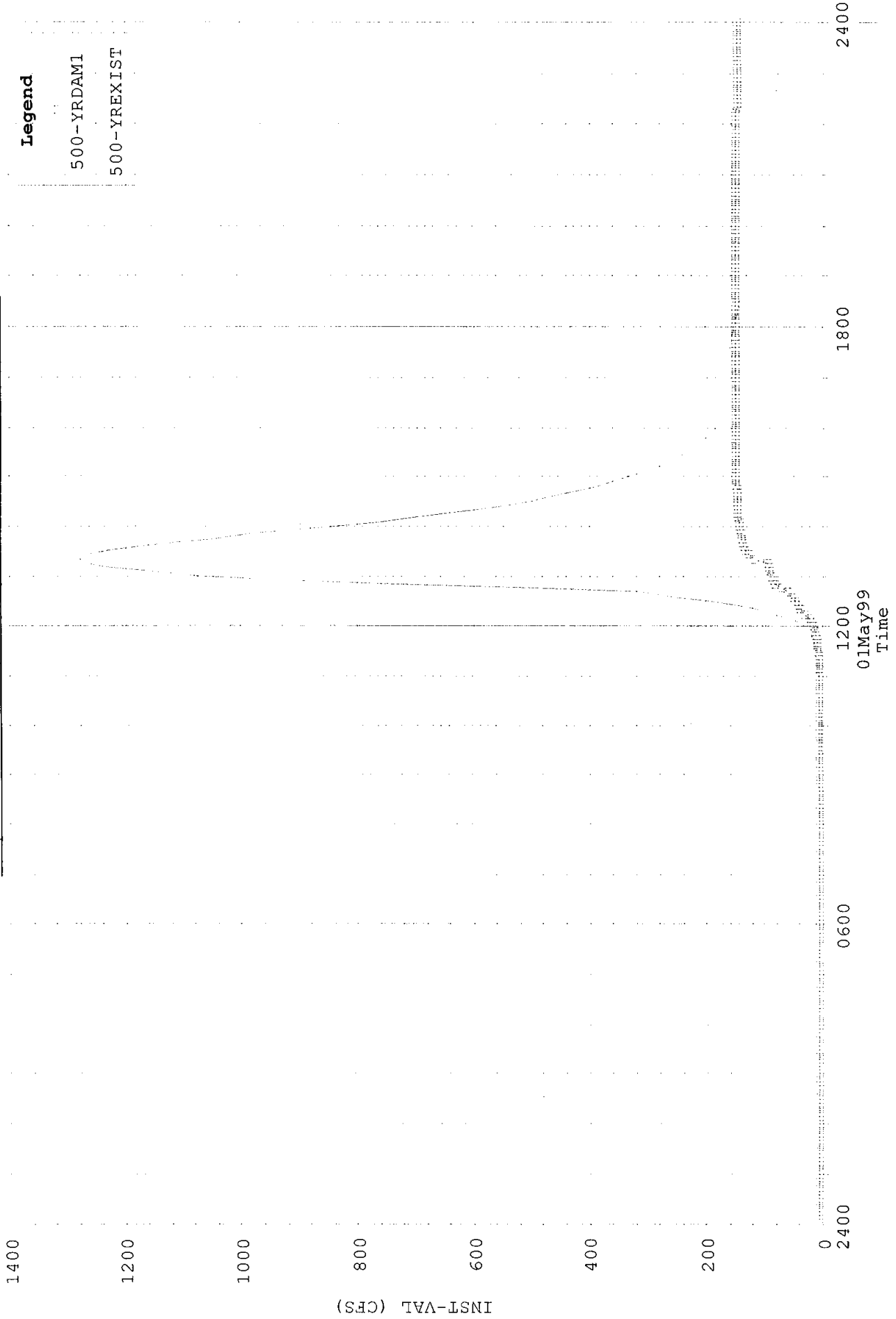
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3001E+03 EXCESS= .0000E+00 OUTFLOW= .2989E+03 BASIN STORAGE= .1631E+01 PERCENT ERROR= -.2

* NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2EV500.DSS
Pointer Utilization: .25
Number of Records: 17
File Size: 42.3 Kbytes
Percent Inactive: .00

500-YR DAM1, EXIST FLOW

Proposed Damsite Hydrographs: Evans Creek Block N



Legend

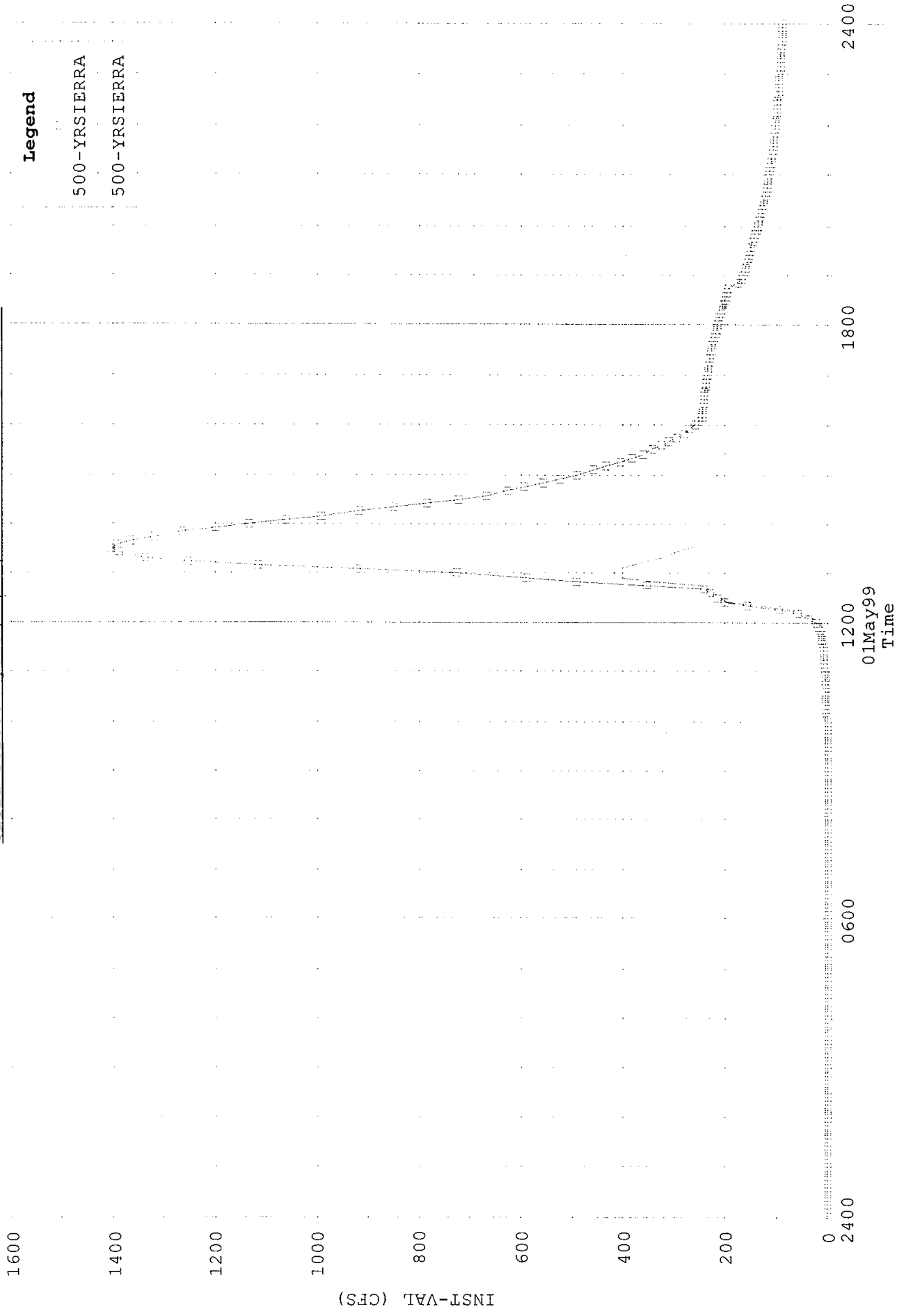
500-YRDAM1

500-YREXIST

1200
01May99
Time

500-YR SIERRA FLOW

Sierra Street Hydrographs; Evans Creek Block N



**100-YEAR, 24 HOUR
WITH NRCS DISCHARGE
CURVE AT PROPOSED
DAM**

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID    PHASE II SUB-AREAS
2         ID
3         ID    EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID    100-YEAR 24 HOUR EVENT                DATE : MAY 1999
5         ID    NIMBUS ENGINEERS, RENO, NEVADA         FILE NAME: 2sc100.DAT
6         ID
7         ID
8         ID    THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID    *****
10        ID    GONI CANYON                            Nimbus Job #: 9606      *
11        ID    Prepared by Nimbus Engineers           Date:  August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID    *****
14        ID
15        ID    The 100-yr PH card rainfall depths were determined from the
16        ID    Precipitation Frequency Study of the United States, NOAA Atlas 14,
17        ID    Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation
18        ID    events and the methodology per the Draft Washoe County Hydrologic
19        ID    Criteria and Design Manual.
20        ID
21        ID    *****
22        ID    THIS MODEL IS FOR PROPOSED (PLAN 1) WITHOUT THE 30" CMP RISER AND *
23        ID    EXISTING CONDITIONS (PLAN 2) *
24        ID    *****
25        IT    5 01MAY99    0005    288
26        IO    4      0
27        JP    2
28        *    DEPTH AREA REDUCTION FACTOR FROM NOAA
29        JR    PREC    0.99
30        *
31        KK    E1 SUBBASIN E1
32        BA    0.772
33        PH           0.48    0.87    1.45    1.62    1.76    2.03    2.50    2.97
34        LS           76
35        UD    0.79
36        ZW    A=100-YR C=FLOW F=E1FLOW
37        *
38        KK    E2 SUB-BASIN E2
39        BA    1.2
40        PH           0.48    0.87    1.45    1.57    1.66    1.85    2.29    2.73
41        LS           77
42        UD    0.99
43        ZW    A=100-YR C=FLOW F=E2FLOW
44        *
45        KK    CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
46        * KO    2
47        HC    2
48        ZW    A=100-YR C=FLOW F=E1+E2
49        *

```

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44      KK      RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
45      RD      10800   0.026   0.07           TRAP      15      2
          *

46      KK      E3 SUBBASIN E3
47      BA      1.51
48      PH              0.48   0.87   1.45   1.57   1.66   1.85   2.33   2.80
49      LS              81
50      UD      1.24
51      ZW      A=100-YR C=FLOW F=E3FLOW
          *

52      KK      CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
53      HC      2
          * ZW A=100-YR C=FLOW F=E1+E2+E3
          *

54      KK      DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
          * KO      2

55      KP      1
56      KM      PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.
57      RS      1      STOR      0
58      SA      3.63   3.71   3.79   3.87   3.95   5.12   6.44   7.97   8.92   9.93
59      SA      10.18  10.45  10.94  11.55  12.67  14.15  15.62
60      SE      795.89  796.33  796.76  797.2  797.64  803.16  808.69  814.22  816.98  819.74
61      SE      820.36  820.97  822.07  823.42  825.87  828.94  832.0
62      SQ      0      13.46  38.09  69.97  107.73  114.35  120.61  126.56  129.43  132.24
63      SQ      218.16  401.01  881.31  1708.7  3654.6  6920.1  11031.7
64      ZW      A=100-YR C=FLOW F=DAMFLOW
65      ZW      C=STOR F=DAMSTOR
66      ZW      C=STAGE F=DAMSTAGE
          *
          * KP      1
          * KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spill
          * RS      1      STOR      0
          * SA      1.18   1.21   1.23   1.27   1.32   1.41   1.50   1.59   1.70   1.
          * SA      2.83   3.91   3.97   4.72   6.68   9.07   10.37  11.09  12.75  15.
          * SE      775.4   775.7   776.0   776.4   777.0   778.0   779.0   780.0   781.0   783
          * SE      790     796     796.3   800.4   808.69  816.98  820.36  822.10  825.87  832
          * SQ      5.4     12.6    21.4    35     40.8    49.9    57.5    64.3    70.4    81
          * SQ      92.1    95.43   112.2    134    145    155.4   244.5   907.5   3680.8  110
          * ZW A=100-YR C=FLOW F=30CMP
          * ZW C=STOR F=30CMP
          * ZW C=STAGE F=30CMP
          *
          * KO      2

67      KP      2
68      KM      EXISTING CONDITIONS (NO RESERVOIR)
69      RN
70      ZW      A=100-YR B=EXIST C=FLOW F=EXISTFLOW
          *
    
```

LINE	ID	1	2	3	4	5	6	7	8	9	10
71	KK	RCH2 ROUTE TO CP3									
72	KM	ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3									
73	KM	HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES									
74	RD	4200	0.023	0.09		TRAP	20	2			
	*										
75	KK	E4 SUBBASIN E4									
76	BA	0.58									
77	PH		0.48	0.87	1.45	1.55	1.63	1.79	2.20	2.60	
78	LS		78								
79	UD	0.57									
80	ZW	A=100-YR C=FLOW F=E4FLOW									
	*										
81	KK	CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3									
	* KO	2									
82	HC	2									
83	ZW	A=100-YR C=FLOW F=E4+CP2									
	*										
84	KK	RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S									
85	RD	3400	0.019	0.045		TRAP	20	2			
	*										
86	KK	E5 SUB-BASIN E5									
87	BA	0.31									
88	PH		0.47	0.85	1.41	1.54	1.63	1.83	2.14	2.44	
89	LS		80								
90	UD	0.5									
91	ZW	A=100-YR C=FLOW F=E5FLOW									
	*										
92	KK	CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH									
	* KO	2									
93	KM	FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET									
94	HC	2									
	* ZW	A=100-YR C=FLOW F=SIERRA STREET									
	*										
95	KK	SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET									
96	KM	STORAGE ROUTING AT SIERRA STREET									
97	RS	1	STOR	0							
98	SA	0	0.51	0.76	1.2	1.88	2.59	3.13			
99	SE	587	594	596	598	600	602	604			
100	SQ	0	193	211	229	245	637	1000			
101	ZW	A=100-YR C=FLOW F=SIERRA STREET									
	*										
102	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

29	E1	
	.	
	.	
35	.	E2
	.	.
	.	.
41	CP1.....	
	V	
	V	
44	RCH1	
	.	
	.	
46	.	E3
	.	.
	.	.
52	CP2.....	
	V	
	V	
54	DAM1	
	V	
	V	
71	RCH2	
	.	
	.	
75	.	E4
	.	.
	.	.
81	CP3.....	
	V	
	V	
84	RCH3	
	.	
	.	
86	.	E5
	.	.
	.	.
92	CP4.....	
	V	
	V	
95	SIERRA	

***) RUNOFF ALSO COMPUTED AT THIS LOCATION


```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                    *
*
* RUN DATE 07/07/1999 TIME 15:55:37 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS    *
* HYDROLOGIC ENGINEERING CENTER   *
*   609 SECOND STREET             *
*   DAVIS, CALIFORNIA 95616       *
*   (916) 756-1104                *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
100-YEAR 24 HOUR EVENT             DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA      FILE NAME: 2sc100.DAT

```

```

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
*****
GONI CANYON                        Nimbus Job #: 9606      *
Prepared by Nimbus Engineers        Date: August 1996   *
                                     File Name: GONI-5.DAT *
*****

```

The 100-yr PH card rainfall depths were determined from the Precipitation Frequency Study of the United States, NOAA Atlas 14, Volume 1 - Semi-Arid Southwest United States; 2-year 24 hour precipitation events and the methodology per the Draft Washoe County Hydrologic Criteria and Design Manual.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITHOUT THE 30" CMP RISER AND *
EXISTING CONDITIONS (PLAN 2)                                       *
*****

```

26 IO

OUTPUT CONTROL VARIABLES

```

IPRNT      4  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE

```

IT

HYDROGRAPH TIME DATA

```

NMIN      5  MINUTES IN COMPUTATION INTERVAL
IDATE     1MAY99  STARTING DATE
ITIME     0005  STARTING TIME
NQ        288  NUMBER OF HYDROGRAPH ORDINATES
NDDATE    2MAY99  ENDING DATE
NDTIME    0000  ENDING TIME
ICENT     19  CENTURY MARK

```

```

COMPUTATION INTERVAL    .08 HOURS
TOTAL TIME BASE        23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA          SQUARE MILES
PRECIPITATION DEPTH    INCHES
LENGTH, ELEVATION      FEET
FLOW                   CUBIC FEET PER SECOND
STORAGE VOLUME         ACRE-FEET
SURFACE AREA           ACRES
TEMPERATURE            DEGREES FAHRENHEIT

```

JP

MULTI-PLAN OPTION

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
.99

* *
29 KK * E1 * SUBBASIN E1
* *

SUBBASIN RUNOFF DATA

30 BA SUBBASIN CHARACTERISTICS
TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

31 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.48 .87 1.45 1.62 1.76 2.03 2.50 2.97 .00 .00 .00 .00

STORM AREA = .77

32 LS SCS LOSS RATE
STRTL .63 INITIAL ABSTRACTION
CRVNBR 76.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

33 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .79 LAG

UNIT HYDROGRAPH
49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened

Unit: 71, File: 2SC100.DSS

-----DSS---ZWRITE Unit 71; Vers. 3: /100-YR/E1/FLOW/01MAY1999/5MIN/E1FLOW/

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

* *
35 KK * E2 * SUB-BASIN E2
* *

SUBBASIN RUNOFF DATA

36 BA

SUBBASIN CHARACTERISTICS

TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

37 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.48	.87	1.45	1.57	1.66	1.85	2.29	2.73	.00	.00	.00	.00

STORM AREA = 1.20

38 LS

SCS LOSS RATE

STRTL .60 INITIAL ABSTRACTION
 CRVNBR 77.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

39 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .99 LAG

UNIT HYDROGRAPH

61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

-----DSS---ZWRITE Unit 71; Vers. 3: /100-YR/E2/FLOW/01MAY1999/5MIN/E2FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

41 KK

 * *
 * CP1 *
 * *

COMBINE E1 AND E2 HYDROGRAPHS AT CP1

42 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

-----DSS---ZWRITE Unit 71; Vers. 3: /100-YR/CP1/FLOW/01MAY1999/5MIN/E1+E2/
 -----DSS---ZWRITE Unit 71; Vers. 3: /100-YR/CP1/FLOW/01MAY1999/5MIN//

*** **

44 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
 * * *

HYDROGRAPH ROUTING DATA

45 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 10800. CHANNEL LENGTH
 S .0260 SLOPE
 N .070 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 15.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * *
 46 KK * E3 * SUBBASIN E3
 * *

SUBBASIN RUNOFF DATA

17 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

48 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 .48 .87 1.45 1.57 1.66 1.85 2.33 2.80 .00 .00 .00 .00

STORM AREA = 1.51

19 LS SCS LOSS RATE
 STRTL .47 INITIAL ABSTRACTION
 CRVNBR 81.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

50 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 1.24 LAG

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.

7. 6. 6. 5. 5. 4. 4. 3. 3. 3.
 2. 2. 1. 1. 1. 0.

---DSS---ZWRITE Unit 71; Vers. 3: /100-YR/E3/FLOW/01MAY1999/5MIN/E3FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

 * *
 52 KK * CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
 * *

53 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

 * *
 54 KK * DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 * *

* **

55 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.

HYDROGRAPH ROUTING DATA

57 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

58 SA	AREA	3.6	3.7	3.8	3.9	4.0	5.1	6.4	8.0	8.9	9.9
		10.2	10.4	10.9	11.6	12.7	14.1	15.6			
60 SE	ELEVATION	795.89	796.33	796.76	797.20	797.64	803.16	808.69	814.22	816.98	819.74
		820.36	820.97	822.07	823.42	825.87	828.94	832.00			
62 SQ	DISCHARGE	0.	13.	38.	70.	108.	114.	121.	127.	129.	132.
		218.	401.	881.	1709.	3655.	6920.	11032.			

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	1.61	3.23	4.91	6.63	31.60	63.49	103.26	126.55	152.55
ELEVATION	795.89	796.33	796.76	797.20	797.64	803.16	808.69	814.22	816.98	819.74

STORAGE	158.79	165.08	176.84	192.02	221.68	262.83	308.36
ELEVATION	820.36	820.97	822.07	823.42	825.87	828.94	832.00
---DSS---ZWRITE Unit	71; Vers.	1:	/100-YR/DAM1/FLOW/01MAY1999/5MIN/DAMSTAGE/				
---DSS---ZWRITE Unit	71; Vers.	1:	/100-YR/DAM1/STOR/01MAY1999/5MIN/DAMSTAGE/				
---DSS---ZWRITE Unit	71; Vers.	1:	/100-YR/DAM1/STAGE/01MAY1999/5MIN/DAMSTAGE/				

67 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

69 RN NO ROUTING

---DSS---ZWRITE Unit 71; Vers. 1: /100-YR/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
 ---DSS---ZWRITE Unit 71; Vers. 1: /100-YR/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
 ---DSS---ZWRITE Unit 71; Vers. 1: /100-YR/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

 * *
 * RCH2 *
 * *

71 KK ROUTE TO CP3

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
 HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

74 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	4200.	CHANNEL LENGTH
S	.0230	SLOPE
N	.090	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

PLAN 2 INPUT DATA FOR STATION RCH2 ARE SAME AS FOR PLAN 1

 * *
 * E4 *
 * *

75 KK SUBBASIN E4

SUBBASIN RUNOFF DATA

76 BA SUBBASIN CHARACTERISTICS

TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

77 PH

HYDRO-35		DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM								TP-40		TP-49	
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY		
.48	.87	1.45	1.55	1.63	1.79	2.20	2.60	.00	.00	.00	.00		

STORM AREA = .58

78 LS

SCS LOSS RATE

STRTL .56 INITIAL ABSTRACTION
 CRVNBR 78.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

79 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .57 LAG

UNIT HYDROGRAPH

36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

-----DSS---ZWRITE Unit 71; Vers. 1: /100-YR/E4/FLOW/01MAY1999/5MIN/E4FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

81 KK

 * *
 * CP3 *
 * *

COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3

82 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

-----DSS---ZWRITE Unit 71; Vers. 1: /100-YR/CP3/FLOW/01MAY1999/5MIN/E4+CP2/

---DSS---ZWRITE Unit 71; Vers. 1: /100-YR/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

84 KK

 * *
 * RCH3 *
 * *

ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S

HYDROGRAPH ROUTING DATA

85 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 3400. CHANNEL LENGTH
 S .0190 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER
 Z 2.00 SIDE SLOPE

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

 * *
 86 KK * E5 * SUB-BASIN E5
 * *

SUBBASIN RUNOFF DATA

37 BA SUBBASIN CHARACTERISTICS
 TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

38 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.47	.85	1.41	1.54	1.63	1.83	2.14	2.44	.00	.00	.00	.00

STORM AREA = .31

39 LS SCS LOSS RATE

STRTL	.50	INITIAL ABSTRACTION
CRVNBR	80.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

90 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.50	LAG
------	-----	-----

UNIT HYDROGRAPH
 32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

-----DSS---ZWRITE Unit 71; Vers. 1: /100-YR/E5/FLOW/01MAY1999/5MIN/E5FLOW/

*** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

```

*****
*           *
92 KK      *   CP4   *   COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
*           *
*****

```

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

```

94 HC      HYDROGRAPH COMBINATION
           ICOMP          2  NUMBER OF HYDROGRAPHS TO COMBINE

```

```

*****
*           *
95 KK      *   SIERRA *   STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
*           *
*****

```

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

```

97 RS      STORAGE ROUTING
           NSTPS          1  NUMBER OF SUBREACHES
           ITYP          STOR TYPE OF INITIAL CONDITION
           RSVRIC        .00 INITIAL CONDITION
           X              .00 WORKING R AND D COEFFICIENT

98 SA      AREA          .0      .5      .8      1.2      1.9      2.6      3.1

99 SE      ELEVATION     587.00  594.00  596.00  598.00  600.00  602.00  604.00

100 SQ     DISCHARGE     0.      193.   211.   229.   245.   637.   1000.

```

COMPUTED STORAGE-ELEVATION DATA

```

STORAGE      .00      1.19      2.45      4.40      7.45      11.90      17.61
ELEVATION    587.00  594.00  596.00  598.00  600.00  602.00  604.00
-----DSS---ZWRITE Unit 71; Vers. 1: /100-YR/SIERRA/FLOW/01MAY1999/5MIN/SIERRA STREET/

```

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

```

-----DSS---ZWRITE Unit 71; Vers. 1: /100-YR/SIERRA/FLOW/01MAY1999/5MIN/EXISTFLOW/

```

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					.99
HYDROGRAPH AT	E1	.77	1	FLOW	215.
				TIME	12.83
			2	FLOW	215.
				TIME	12.83
HYDROGRAPH AT	E2	1.20	1	FLOW	264.
				TIME	13.08
			2	FLOW	264.
				TIME	13.08
COMBINED AT	CP1	1.97	1	FLOW	469.
				TIME	12.92
			2	FLOW	469.
				TIME	12.92
ROUTED TO	RCH1	1.97	1	FLOW	475.
				TIME	13.25
			2	FLOW	475.
				TIME	13.25
HYDROGRAPH AT	E3	1.51	1	FLOW	372.
				TIME	13.33
			2	FLOW	372.
				TIME	13.33
COMBINED AT	CP2	3.48	1	FLOW	847.
				TIME	13.25
			2	FLOW	847.
				TIME	13.25
ROUTED TO	DAM1	3.48	1	FLOW	124.
				TIME	16.42
			2	FLOW	847.
				TIME	13.25
** PEAK STAGES IN FEET **					
			1	STAGE	812.24
				TIME	16.42
			2	STAGE	.00
				TIME	.00
ROUTED TO	RCH2	3.48	1	FLOW	124.
				TIME	16.67
			2	FLOW	857.
				TIME	13.33
HYDROGRAPH AT	E4	.58	1	FLOW	196.
				TIME	12.58
			2	FLOW	196.
				TIME	12.58
COMBINED AT	CP3	4.06	1	FLOW	206.
				TIME	13.08
			2	FLOW	921.
				TIME	13.33

ROUTED TO	RCH3	4.06	1	FLOW	205.
				TIME	13.25
			2	FLOW	915.
				TIME	13.42

HYDROGRAPH AT	E5	.31	1	FLOW	119.
				TIME	12.50
			2	FLOW	119.
				TIME	12.50

2 COMBINED AT	CP4	4.37	1	FLOW	307.
				TIME	12.67
			2	FLOW	941.
				TIME	13.42

ROUTED TO	SIERRA	4.37	1	FLOW	229.
				TIME	13.33
			2	FLOW	880.
				TIME	13.67

** PEAK STAGES IN FEET **

1	STAGE	598.06
	TIME	13.33
2	STAGE	603.34
	TIME	13.67

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	475.31	795.00	.87	5.00	475.31	795.00	.87
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9298E+02 EXCESS= .0000E+00 OUTFLOW= .9108E+02 BASIN STORAGE= .2794E+01 PERCENT ERROR= -1.0

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	475.31	795.00	.87	5.00	475.31	795.00	.87
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9298E+02 EXCESS= .0000E+00 OUTFLOW= .9108E+02 BASIN STORAGE= .2794E+01 PERCENT ERROR= -1.0

FOR PLAN = 1 RATIO= .00

RCH2	MANE	5.00	124.43	1000.00	.60	5.00	124.43	1000.00	.60
------	------	------	--------	---------	-----	------	--------	---------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1152E+03 EXCESS= .0000E+00 OUTFLOW= .1115E+03 BASIN STORAGE= .3883E+01 PERCENT ERROR= -1.2

FOR PLAN = 2 RATIO= .00

RCH2	MANE	5.00	856.92	800.00	.96	5.00	856.92	800.00	.96
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1794E+03 EXCESS= .0000E+00 OUTFLOW= .1778E+03 BASIN STORAGE= .2258E+01 PERCENT ERROR= -1.4

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	205.41	795.00	.62	5.00	205.41	795.00	.62
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1369E+03 EXCESS= .0000E+00 OUTFLOW= .1349E+03 BASIN STORAGE= .2193E+01 PERCENT ERROR= -1.1

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	914.76	805.00	.93	5.00	914.76	805.00	.93
------	------	------	--------	--------	-----	------	--------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2032E+03 EXCESS= .0000E+00 OUTFLOW= .2023E+03 BASIN STORAGE= .1344E+01 PERCENT ERROR= -1.2

* NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2SC100.DSS

Pointer Utilization: .25

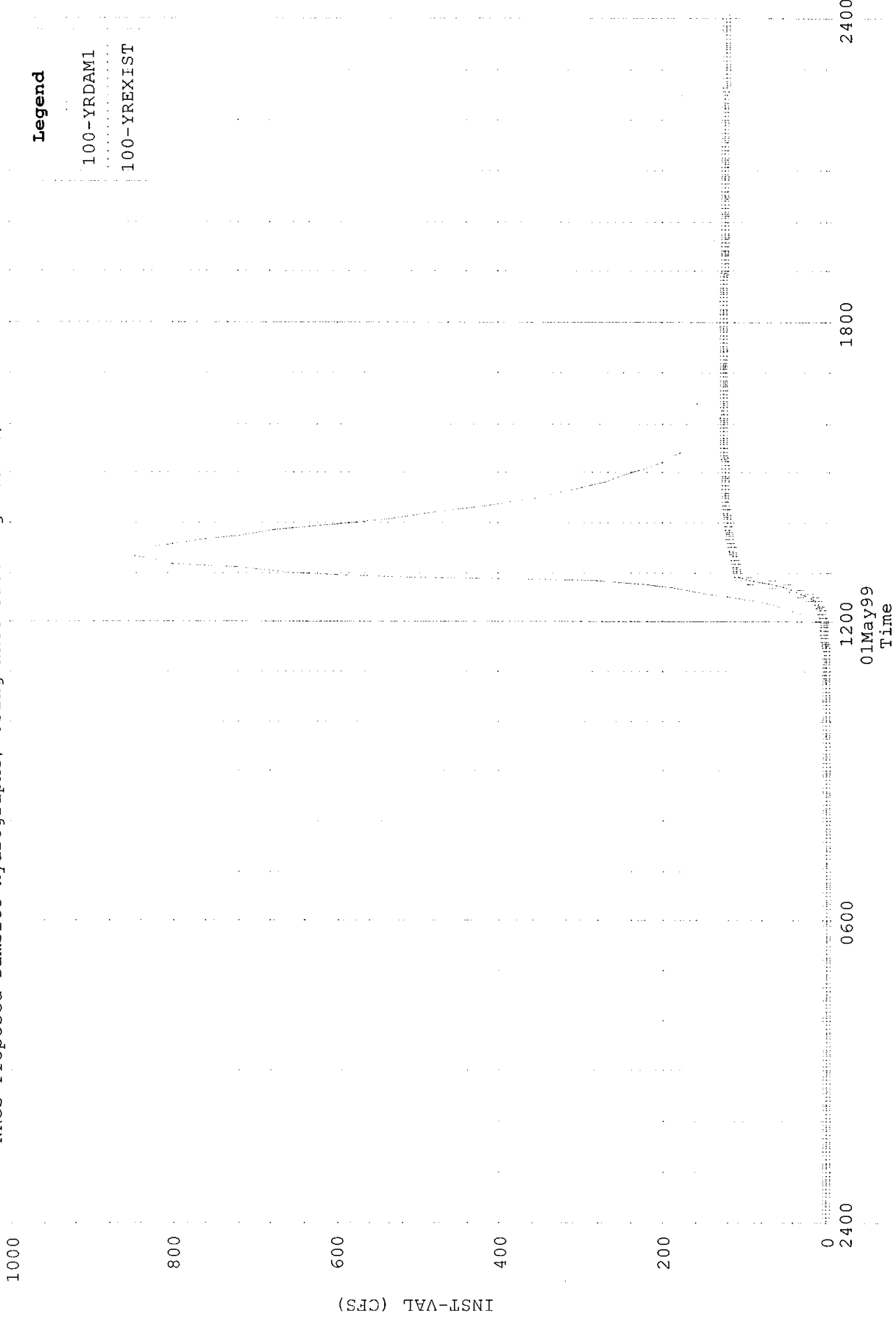
Number of Records: 17

File Size: 41.8 Kbytes

Percent Inactive: .00

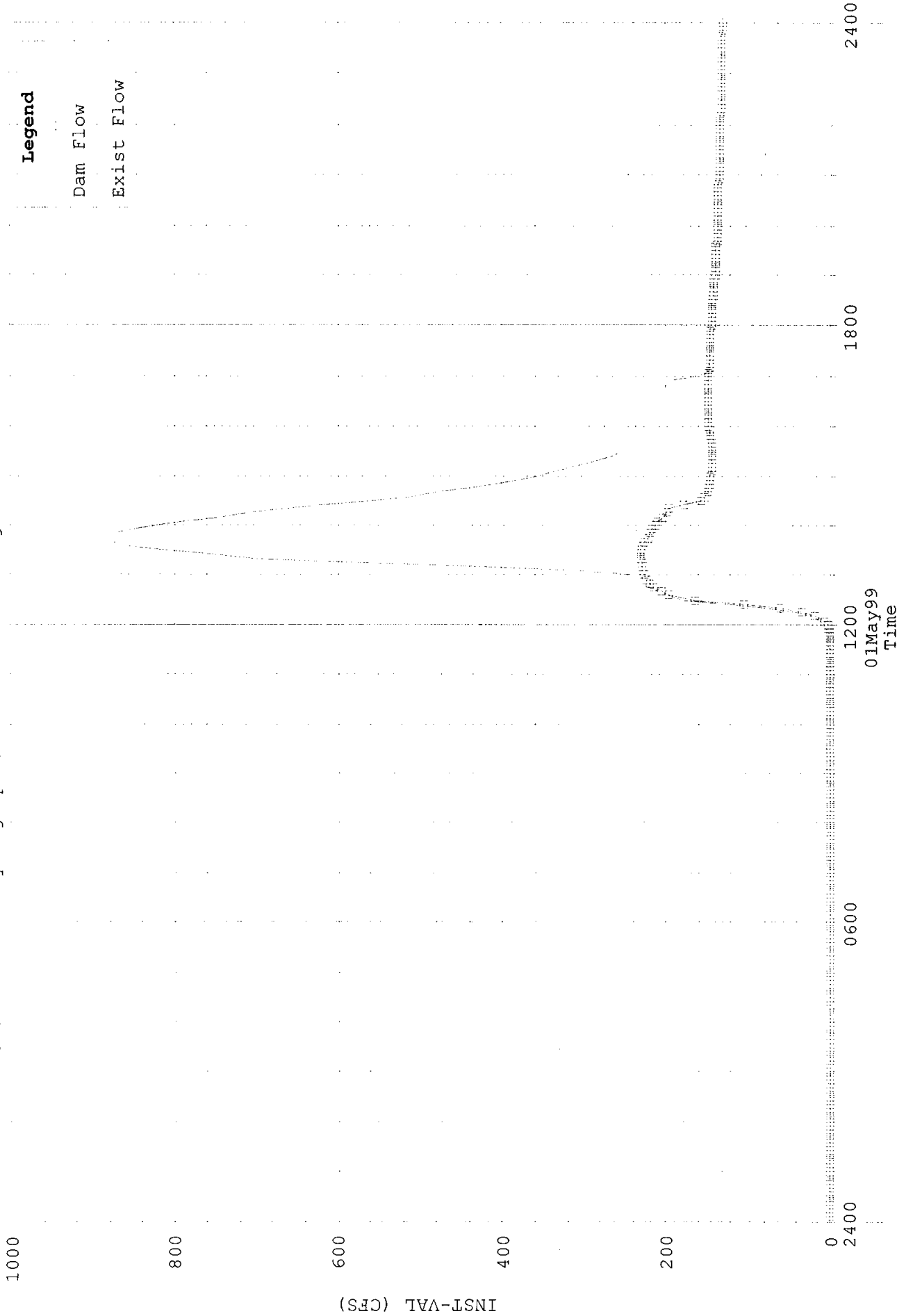
100-YR DAM1, EXIST FLOW

NRCS Proposed Damsite Hydrographs; Using NRCS Discharge Curve; Evans Creek Block N



100-YR SIERRA FLOW

Sierra Street Hydrographs; With NRCS Discharge Curve; Evans Creek Block N



1986-AIRPORT DATA, 4-DAY

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*DIAGRAM
1         ID      PHASE II SUB-AREAS
2         ID
3         ID      EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID      1986 STORM EVENT; AIRPORT GAGE DATA    DATE : MAY 1999
5         ID      NIMBUS ENGINEERS, RENO, NEVADA          FILE NAME: 2PIAIR.DAT
6         ID
7         ID
8         ID      THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID      *****
10        ID      GONI CANYON                               Nimbus Job #: 9606      *
11        ID      Prepared by Nimbus Engineers             Date: August 1996    *
12        ID                                             File Name: GONI-5.DAT *
13        ID      *****
14        ID
15        ID      The airport PI card distribution is the raingage data
16        ID      from the 1986 storm event.  Precipitation depths are from 0100
17        ID      2/14/86 to 0300 on 2/20/86.
18        ID
19        ID      *****
20        ID      THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
21        ID      EXISTING CONDITIONS (PLAN 2)                                *
22        ID      *****
23        ID      CURVE NUMBERS WERE MODIFIED FROM AMC-II CONDITIONS TO AMC-III CONDITIONS
24        ID      IN ORDER TO MODEL A SATURATED WATERSHED.  REFERENCE: McCUEN, R.H. (1982);
25        ID      "A GUIDE TO HYDROLOGIC ANALYSIS USING SCS METHODS", PRENTICE-HALL, INC.
26        ID
27        ID
28        IT      15 14FEB86      0100          20FEB86      0300
29        IO      4      0
30        IN      60 14FEB86      0100
31        PG      AIR      0
32        PI      .001 .001 .01 .01 .001 .0 .001 .001 .001 .001
33        PI      .001 0 0 .001 .01 .05 .18 .16 .11 .001
34        PI      .001 .001 0 0 .001 .02 .04 .16 .02 .05
35        PI      .01 .001 .001 .01 .07 .01 .01 .001 .01 .01
36        PI      .07 .05 .03 .02 .01 .02 .1 .03 .05 .07
37        PI      .001 0 0 0 0 0 0 0 0 0 .001
38        PI      .001 0 .001 .01 .01 .01 .01 .02 .01 .02
39        PI      .001 .01 .08 .05 .05 .06 .02 .03 .03 .01
40        PI      .001 .01 .05 .03 .02 .01 .06 .06 .04 .04
41        PI      .13 .09 .03 .04 .01 .01 .001 .001 .001 .01
42        PI      .001 .001 .01 .001 .01 .01 .02 .02 .001 .001
43        PI      .01 .06 .02 .05 .07 .08 .09 .04 .12 .21
44        PI      .13 .13 .12 .12 .03 .02 .02 .001 .001 .03
45        PI      .001 .02 .01 .04 .04 .02 .02 .04 .09 .08
46        PI      .02 .02 .03 .03 .02 .01 .001
47        JP      2
*         DEPTH AREA REDUCTION FACTOR FROM NOAA
48        JR      PREC      0.99
*

```


LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

49 KK E1 SUBBASIN E1
 50 BA 0.772
 51 PR AIR
 52 PW 1.0
 * LS 76
 53 LS 91
 54 UD 0.79
 * ZW A=1986 C=FLOW E=4HOUR F=E1FLOW
 *

55 KK E2 SUBBASIN E2
 56 BA 1.2
 * LS 77
 57 LS 92
 58 UD 0.99
 * ZW A=1986 C=FLOW E=4HOUR F=E2FLOW
 *

59 KK CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
 60 HC 2
 61 ZW A=1986 C=FLOW E=4HOUR F=E1+E2
 *

62 KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
 63 RD 10800 0.026 0.07 TRAP 15 2
 *

64 KK E3 SUB-BASIN E3
 65 BA 1.51
 * LS 81
 66 LS 95
 67 UD 1.24
 * ZW A=1986 C=FLOW E=4HOUR F=E3FLOW
 *

68 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
 69 HC 2
 * ZW A=1986 C=FLOW E=4HOUR F=E1+E2+E3
 *

70 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
 * KP 1
 * KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.
 * RS 1 STOR 0
 * SA .1354 .9137 1.954 3.63 3.71 3.79 3.87 3.95 5.12 6.
 * SA 7.97 8.92 9.93 10.18 10.45 10.94 11.55 12.67 14.15 15.
 * SE 765 775 785 795.89 796.33 796.76 797.2 797.64 803.16 808.
 * SE814.22 816.98 819.74 820.36 820.97 822.07 823.42 825.87 828.94 832
 * SQ 0 0 0 0 13.46 38.09 69.97 107.73 114.35 120.
 * SQ126.56 129.43 132.24 218.16 401.01 881.31 1708.7 3654.6 6920.1 11031
 * ZW A=1986 C=FLOW E=4HOUR F=DAMFLOW
 * ZW C=STOR E=4HOUR F=DAMSTOR
 * ZW C=STAGE E=4HOUR F=DAMSTAGE
 *

LINE	ID	1	2	3	4	5	6	7	8	9	10
71	KP	1									
72	KM	PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa									
73	RS	1	STOR	0							
74	SA	1.18	1.21	1.23	1.27	1.32	1.41	1.50	1.59	1.70	1.91
75	SA	2.83	3.91	3.97	4.72	6.68	9.07	10.37	11.09	12.75	15.70
76	SE	775.4	775.7	776.0	776.4	777.0	778.0	779.0	780.0	781.0	783.0
77	SE	790	796	796.3	800.4	808.69	816.98	820.36	822.10	825.87	832.0
78	SQ	5.4	12.6	21.4	35	40.8	49.9	57.5	64.3	70.4	81.3
79	SQ	92.1	95.43	112.2	134	145	155.4	244.5	907.5	3680.8	11060
80	ZW	A=1986 C=FLOW E=4HOUR F=30CMP									
81	ZW	C=STOR E=4HOUR F=30CMP									
82	ZW	C=STAGE E=4HOUR F=30CMP									
		*									
83	KP	2									
84	KM	EXISTING CONDITIONS (NO RESERVOIR)									
85	RN										
86	ZW	A=1986 B=EXIST C=FLOW E=4HOUR F=EXISTFLOW									
		*									
87	KK	RCH3 ROUTE TO CP3									
88	KM	ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3									
89	KM	HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES									
90	RD	4200	0.023	0.09		TRAP	20	2			
		*									
91	KK	E4 SUBBASIN E4									
92	BA	0.58									
		* LS 78									
93	LS	93									
94	UD	0.57									
		* ZW A=1986 C=FLOW E=4HOUR F=E4FLOW									
		*									
95	KK	CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3									
96	HC	2									
97	ZW	A=1986 C=FLOW E=4HOUR F=E4+CP2									
		*									
98	KK	RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S									
99	RD	3400	0.019	0.045		TRAP	20	2			
		*									
100	KK	E5 SUB-BASIN E5									
101	BA	0.31									
		* LS 80									
102	LS	94									
103	UD	0.5									
		* ZW A=1986 C=FLOW E=4HOUR F=E5FLOW									
		*									

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

104 KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH

105 KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

106 HC 2

107 ZW A=1986 C=FLOW E=4HOUR F=CP4

*

108 KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET

109 KM STORAGE ROUTING AT SIERRA STREET

110 RS 1 STOR 0

111 SA 0 0.76 1.2 1.88 2.59 3.13

112 SE 587 596 598 600 602 604

113 SQ 0 211 229 245 637 1000

114 ZW A=1986 C=FLOW F=SIERRA STREET

*

115 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
49	E1	
	.	
55	.	E2
	.	.
59	CP1.....	
	V	
	V	
62	RCH1	
	.	
64	.	E3
	.	.
68	CP2.....	
	V	
	V	
70	DAM1	
	V	
	V	
87	RCH3	
	.	
91	.	E4
	.	.
95	CP3.....	
	V	
	V	
98	RCH3	
	.	
100	.	E5
	.	.
104	CP4.....	
	V	
	V	
108	SIERRA	

**) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*
* RUN DATE 06/30/99 TIME 09:21:56
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
1986 STORM EVENT; AIRPORT GAGE DATA    DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA          FILE NAME: 2PIAIR.DAT

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THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
*****
GONI CANYON                               Nimbus Job #: 9606      *
Prepared by Nimbus Engineers              Date: August 1996   *
                                           File Name: GONI-5.DAT *
*****

```

The airport PI card distribution is the raingage data from the 1986 storm event. Precipitation depths are from 0100 2/14/86 to 0300 on 2/20/86.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND
EXISTING CONDITIONS (PLAN 2)
*****
CURVE NUMBERS WERE MODIFIED FROM AMC-II CONDITIONS TO AMC-III CONDITIONS
IN ORDER TO MODEL A SATURATED WATERSHED. REFERENCE: McCUEN, R.H. (1982);
"A GUIDE TO HYDROLOGIC ANALYSIS USING SCS METHODS", PRENTICE-HALL, INC.

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```

29 IO      OUTPUT CONTROL VARIABLES
           IPRNT      4  PRINT CONTROL
           IPLOT      0  PLOT CONTROL
           QSCAL      0. HYDROGRAPH PLOT SCALE

0 IN      TIME DATA FOR INPUT TIME SERIES
           JXMIN      60  TIME INTERVAL IN MINUTES
           JXDATE     14FEB86  STARTING DATE
           JXTIME     100  STARTING TIME

IT        HYDROGRAPH TIME DATA
           NMIN       15  MINUTES IN COMPUTATION INTERVAL
           IDATE      14FEB86  STARTING DATE
           ITIME      0100  STARTING TIME
           NQ         585  NUMBER OF HYDROGRAPH ORDINATES
           NDDATE     20FEB86  ENDING DATE
           NDTIME     0300  ENDING TIME
           ICENT      19  CENTURY MARK

           COMPUTATION INTERVAL  0.25 HOURS
           TOTAL TIME BASE  146.00 HOURS

```

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES

0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.01	0.02	0.01	0.02	0.02	0.01
0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.03	0.03	0.03	0.03	0.03	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.01	0.00
0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.02	0.01	0.02	0.01	0.01	0.00
0.01	0.00	0.01	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.03	0.03	0.03	0.03	0.03	0.05	0.05	0.05	0.05
0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.01	0.01	0.01	0.01
0.01	0.00	0.01	0.00	0.01	0.00	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.00	0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.01	0.01	0.00	0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.01
0.00	0.00	0 00	0 00	0 00						

UNIT HYDROGRAPH
22 END-OF-PERIOD ORDINATES

63.	201.	403.	513.	508.	428.	310.	205.	142.	101.
69.	49.	33.	23.	16.	11.	8.	5.	4.	3.
2.	0.								

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

```

*****
*
59 KK *      CP1 *      COMBINE E1 AND E2 HYDROGRAPHS AT CP1
*      *
*****

```

```

60 HC      HYDROGRAPH COMBINATION
           ICOMP      2 NUMBER OF HYDROGRAPHS TO COMBINE

```

```

-----DSS---ZOPEN: Existing File Opened, File: 2PIAIR.DSS
Unit: 71; DSS Version: 6-FT
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/14FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/15FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/16FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/17FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/18FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/19FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/20FEB1986/15MIN/E1+E2/
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/14FEB1986/15MIN//
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/15FEB1986/15MIN//
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/16FEB1986/15MIN//
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/17FEB1986/15MIN//

```

-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/18FEB1986/15MIN//
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/19FEB1986/15MIN//
-----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP1/FLOW/20FEB1986/15MIN//

* *
62 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
* *

HYDROGRAPH ROUTING DATA

63 RD MUSKINGUM-CUNGE CHANNEL ROUTING
L 10800. CHANNEL LENGTH
S 0.0260 SLOPE
N 0.070 CHANNEL ROUGHNESS COEFFICIENT
CA 0.00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 15.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

* *
64 KK * E3 * SUB-BASIN E3
* *

SUBBASIN RUNOFF DATA

65 BA SUBBASIN CHARACTERISTICS
TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

0 PT TOTAL STORM STATIONS AIR
0 PW WEIGHTS 1.00
1 PR RECORDING STATIONS AIR
2 PW WEIGHTS 1.00

66 LS SCS LOSS RATE
STRTL 0.11 INITIAL ABSTRACTION
CRVNBR 95.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

7 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 1.24 LAG

0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.01
0.00	0.00	0.00	0.00							

UNIT HYDROGRAPH
27 END-OF-PERIOD ORDINATES

47.	144.	301.	457.	530.	529.	466.	382.	273.	198.
147.	111.	82.	61.	45.	33.	25.	19.	14.	10.
8.	6.	5.	3.	2.	1.	0.			

* *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

```

*****
*           *
68 KK      *   CP2   *   COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
*           *
*****
  
```

```

69 HC      HYDROGRAPH COMBINATLON
           ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE
  
```

```

*****
*           *
70 KK      *   DAM1  *   PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
*           *
*****
  
```

*** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

```

71 KP      PLAN 1 FOR STATION   DAM1           PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
           PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa
  
```

HYDROGRAPH ROUTING DATA

```

73 RS      STORAGE ROUTING
           NSTPS           1  NUMBER OF SUBREACHES
           ITYP           STOR  TYPE OF INITIAL CONDITION
           RSVRIC         0.00  INITIAL CONDITION
           X              0.00  WORKING R AND D COEFFICIENT
  
```

74 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7

76 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

78 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.
		92.	95.	112.	134.	145.	155.	245.	908.	3681.	11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	0.36	0.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

```

----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/14FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/15FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/16FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/17FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/18FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/19FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/FLOW/20FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/14FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/15FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/16FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/17FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/18FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/19FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/DAM1/STOR/20FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/14FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/15FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/16FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/17FEB1986/15MIN/30CMP/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/18FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/19FEB1986/15MIN/30CMP/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/DAM1/STAGE/20FEB1986/15MIN/30CMP/
    
```

*** **

33 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

85 RN NO ROUTING

```

----DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/14FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/15FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/16FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/17FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/18FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/19FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/FLOW/20FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/14FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/15FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/16FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/17FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/18FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/19FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 2: /1986/EXIST/STOR/20FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/14FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/15FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/16FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/17FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/18FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/19FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/EXIST/STAGE/20FEB1986/15MIN/EXISTFLOW/
    
```

*** **

```

*****
*
*
87 KK      RCH3 *      ROUTE TO CP3
*
*
*****

```

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

```

90 RD      MUSKINGUM-CUNGE CHANNEL ROUTING
           L      4200.  CHANNEL LENGTH
           S      0.0230 SLOPE
           N      0.090  CHANNEL ROUGHNESS COEFFICIENT
           CA     0.00   CONTRIBUTING AREA
           SHAPE  TRAP   CHANNEL SHAPE
           WD     20.00  BOTTOM WIDTH OR DIAMETER
           Z      2.00   SIDE SLOPE

```

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

```

*****
*
*
91 KK      E4 *      SUBBASIN E4
*
*
*****

```

SUBBASIN RUNOFF DATA

```

92 BA      SUBBASIN CHARACTERISTICS
           TAREA   0.58  SUBBASIN AREA

```

PRECIPITATION DATA

```

0 PT      TOTAL STORM STATIONS   AIR
8 PW      WEIGHTS                 1.00

```

```

51 PR      RECORDING STATIONS     AIR
52 PW      WEIGHTS                 1.00

```

```

53 LS      SCS LOSS RATE
           STRTL   0.15  INITIAL ABSTRACTION
           CRVNBR  93.00  CURVE NUMBER
           RTIMP   0.00  PERCENT IMPERVIOUS AREA

```

```

94 UD      SCS DIMENSIONLESS UNITGRAPH
           TLAG    0.57  LAG

```

PRECIPITATION STATION DATA

STATION	TOTAL	AVG. ANNUAL	WEIGHT
AIR	4.41	0.00	1.00

TEMPORAL DISTRIBUTIONS

STATION	AIR, WEIGHT = 1.00								
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.01	0.01	0.01	0.01	0.05	0.05	0.05	0.04	0.04	0.04
0.04	0.04	0.03	0.03	0.03	0.03	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.04	0.04
0.04	0.04	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.02	0.02
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.02	0.02	0.02	0.02	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.02	0.03	0.02	0.02	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.02
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.02
0.02	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.01	0.02	0.01
0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.03	0.03	0.03	0.03	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.01	0.00
0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.02	0.01	0.02	0.01	0.01	0.00
0.01	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.03	0.03	0.03	0.03	0.05	0.05	0.05	0.05
0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
0.03	0.03	0.03	0.03	0.03	0.03	0.01	0.01	0.01	0.01
0.01	0.00	0.01	0.00	0.01	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.00	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.01	0.01	0.00	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.01
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

105. 338. 399. 298. 157. 89. 50. 28. 15. 8.
5. 3. 1.

*** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

* **

* *
95 KK * CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
* *

96 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/14FEB1986/15MIN/E4+CP2/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/15FEB1986/15MIN/E4+CP2/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/16FEB1986/15MIN/E4+CP2/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/17FEB1986/15MIN/E4+CP2/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/18FEB1986/15MIN/E4+CP2/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/19FEB1986/15MIN/E4+CP2/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/20FEB1986/15MIN/E4+CP2/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/14FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/15FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/16FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/17FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/18FEB1986/15MIN/EXISTFLOW/
---DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/19FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP3/FLOW/20FEB1986/15MIN/EXISTFLOW/

* **

* *
98 KK * RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
* *

HYDROGRAPH ROUTING DATA

99 RD MUSKINGUM-CUNGE CHANNEL ROUTING
L 3400. CHANNEL LENGTH
S 0.0190 SLOPE
N 0.045 CHANNEL ROUGHNESS COEFFICIENT
CA 0.00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 20.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.02	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.01	0.02	0.01	0.02	0.02	0.01
0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.03	0.03	0.03	0.03	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.01	0.00
0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.02	0.01	0.02	0.01	0.01	0.00
0.01	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.01	0.01
0.01	0.01	0.03	0.03	0.03	0.03	0.05	0.05	0.05	0.05
0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
0.03	0.03	0.03	0.03	0.03	0.03	0.01	0.01	0.01	0.01
0.01	0.00	0.01	0.00	0.01	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.00	0.00
0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.00	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.01	0.01	0.00	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.01
0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.00	0.01

UNIT HYDROGRAPH
12 END-OF-PERIOD ORDINATES

75. 225. 225. 135. 68. 35. 19. 10. 5. 3.
1. 0.

* *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

*** **

14 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH

FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET

16 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

----DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/14FEB1986/15MIN/CP4/
 ----DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/15FEB1986/15MIN/CP4/
 --DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/16FEB1986/15MIN/CP4/

```

----DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/17FEB1986/15MIN/CP4/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/18FEB1986/15MIN/CP4/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/19FEB1986/15MIN/CP4/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/CP4/FLOW/20FEB1986/15MIN/CP4/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/14FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/15FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/16FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/17FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/18FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/19FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 7: /1986/CP4/FLOW/20FEB1986/15MIN/EXISTFLOW/

```

```

*****
*
108 KK * SIERRA * STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
*
*****

```

STORAGE ROUTING AT SIERRA STREET

HYDROGRAPH ROUTING DATA

```

110 RS STORAGE ROUTING
      NSTPS          1 NUMBER OF SUBREACHES
      ITYP           STOR TYPE OF INITIAL CONDITION
      RSVRIC         0.00 INITIAL CONDITION
      X              0.00 WORKING R AND D COEFFICIENT

11 SA      AREA      0.0      0.8      1.2      1.9      2.6      3.1

112 SE     ELEVATION 587.00   596.00   598.00   600.00   602.00   604.00

13 SQ     DISCHARGE  0.      211.    229.    245.    637.    1000.

```

COMPUTED STORAGE-ELEVATION DATA

```

STORAGE      0.00      2.28      4.22      7.28      11.73      17.44
ELEVATION    587.00   596.00   598.00   600.00   602.00   604.00
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/14FEB1986/15MIN/SIERRA STREET/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/15FEB1986/15MIN/SIERRA STREET/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/16FEB1986/15MIN/SIERRA STREET/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/17FEB1986/15MIN/SIERRA STREET/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/18FEB1986/15MIN/SIERRA STREET/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/19FEB1986/15MIN/SIERRA STREET/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/20FEB1986/15MIN/SIERRA STREET/

```

PLAN 2 INPUT DATA FOR STATION SIERRA ARE SAME AS FOR PLAN 1

```

----DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/14FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/15FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/16FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/17FEB1986/15MIN/EXISTFLOW/
----DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/18FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/19FEB1986/15MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 2: /1986/SIERRA/FLOW/20FEB1986/15MIN/EXISTFLOW/

```

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	
					0.99
HYDROGRAPH AT	E1	0.77	1	FLOW	83.
				TIME	120.50
			2	FLOW	83.
				TIME	120.50
HYDROGRAPH AT	E2	1.20	1	FLOW	124.
				TIME	120.75
			2	FLOW	124.
				TIME	120.75
COMBINED AT	CP1	1.97	1	FLOW	206.
				TIME	120.50
			2	FLOW	206.
				TIME	120.50
ROUTED TO	RCH1	1.97	1	FLOW	207.
				TIME	121.00
			2	FLOW	207.
				TIME	121.00
HYDROGRAPH AT	E3	1.51	1	FLOW	152.
				TIME	121.00
			2	FLOW	152.
				TIME	121.00
COMBINED AT	CP2	3.48	1	FLOW	359.
				TIME	121.00
			2	FLOW	359.
				TIME	121.00
ROUTED TO	DAM1	3.48	1	FLOW	143.
				TIME	125.75
			2	FLOW	359.
				TIME	121.00
** PEAK STAGES IN FEET **					
			1	STAGE	807.25
				TIME	125.75
			2	STAGE	0.00
				TIME	0.00
ROUTED TO	RCH3	3.48	1	FLOW	143.
				TIME	126.00
			2	FLOW	357.
				TIME	121.25
HYDROGRAPH AT	E4	0.58	1	FLOW	68.
				TIME	120.25
			2	FLOW	68.
				TIME	120.25
COMBINED AT	CP3	4.06	1	FLOW	183.
				TIME	124.00
			2	FLOW	408.
				TIME	121.00

ROUTED TO	RCH3	4.06	1	FLOW	183.
				TIME	124.00
			2	FLOW	408.
				TIME	121.25

DROGRAPH AT	E5	0.31	1	FLOW	37.
				TIME	120.25
			2	FLOW	37.
				TIME	120.25

2 COMBINED AT	CP4	4.37	1	FLOW	206.
				TIME	122.25
			2	FLOW	435.
				TIME	121.25

ROUTED TO	SIERRA	4.37	1	FLOW	206.
				TIME	122.50
			2	FLOW	431.
				TIME	121.25

** PEAK STAGES IN FEET **

1	STAGE	595.78
	TIME	122.50
2	STAGE	600.95
	TIME	121.25

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= 0.00

RCH1	MANE	15.00	206.72	7260.00	3.40	15.00	206.72	7260.00	3.40
------	------	-------	--------	---------	------	-------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3597E+03 EXCESS=0.0000E+00 OUTFLOW=0.3581E+03 BASIN STORAGE=0.2725E+01 PERCENT ERROR= -0.3

FOR PLAN = 2 RATIO= 0.00

RCH1	MANE	15.00	206.72	7260.00	3.40	15.00	206.72	7260.00	3.40
------	------	-------	--------	---------	------	-------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3597E+03 EXCESS=0.0000E+00 OUTFLOW=0.3581E+03 BASIN STORAGE=0.2725E+01 PERCENT ERROR= -0.3

FOR PLAN = 1 RATIO= 0.00

RCH3	MANE	15.00	143.10	7560.00	3.44	15.00	143.10	7560.00	3.44
------	------	-------	--------	---------	------	-------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6429E+03 EXCESS=0.0000E+00 OUTFLOW=0.6403E+03 BASIN STORAGE=0.2762E+01 PERCENT ERROR= 0.0

FOR PLAN = 2 RATIO= 0.00

RCH3	MANE	13.57	358.45	7273.33	3.55	15.00	357.33	7275.00	3.55
------	------	-------	--------	---------	------	-------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6613E+03 EXCESS=0.0000E+00 OUTFLOW=0.6596E+03 BASIN STORAGE=0.2283E+01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.00

RCH3	MANE	9.17	182.65	7445.77	3.45	15.00	182.53	7440.00	3.45
------	------	------	--------	---------	------	-------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7500E+03 EXCESS=0.0000E+00 OUTFLOW=0.7484E+03 BASIN STORAGE=0.1570E+01 PERCENT ERROR= 0.0

FOR PLAN = 2 RATIO= 0.00

RCH3	MANE	7.08	408.75	7273.76	3.55	15.00	408.16	7275.00	3.55
------	------	------	--------	---------	------	-------	--------	---------	------

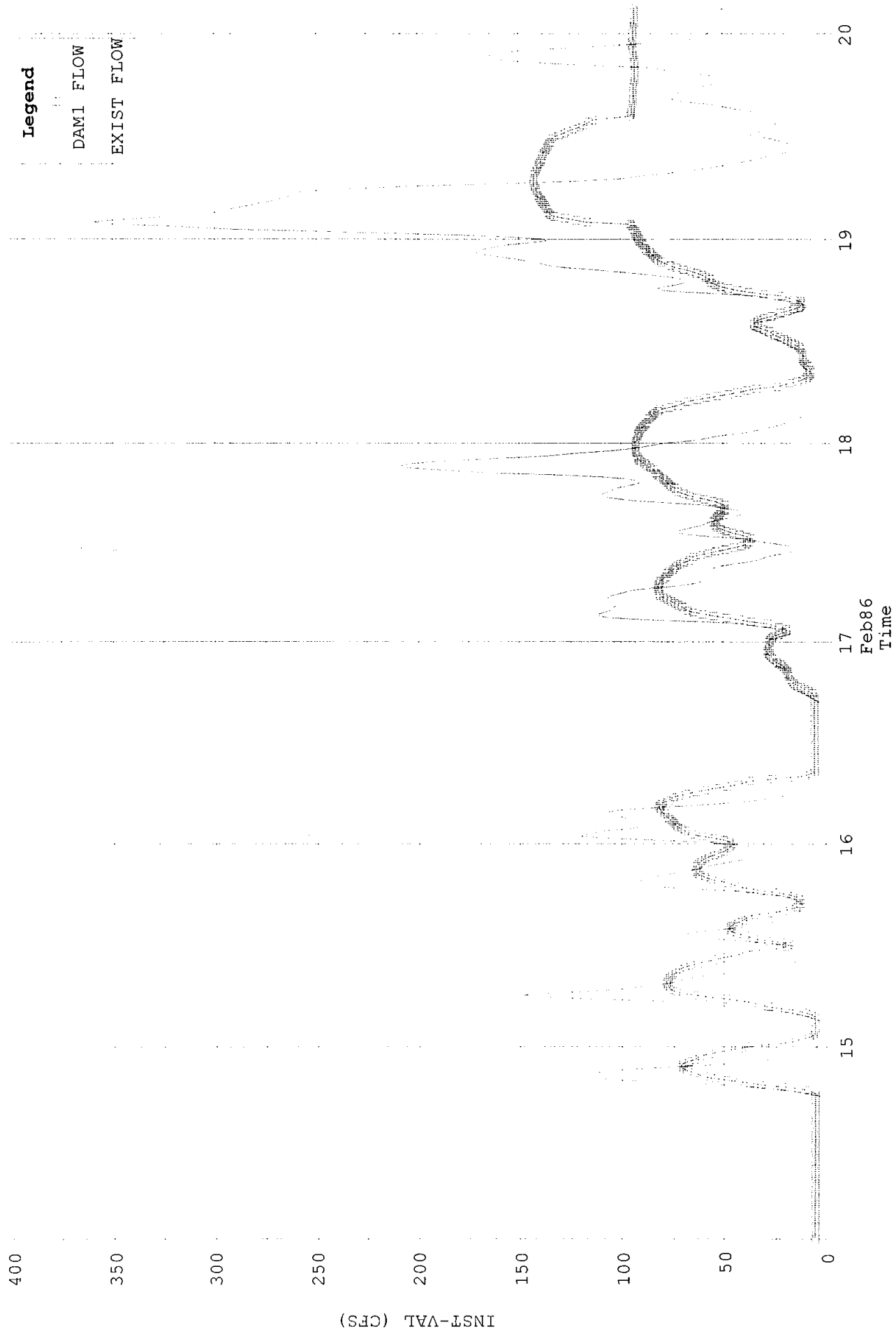
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7703E+03 EXCESS=0.0000E+00 OUTFLOW=0.7692E+03 BASIN STORAGE=0.1350E+01 PERCENT ERROR= 0.0

NORMAL END OF HEC-1 ***

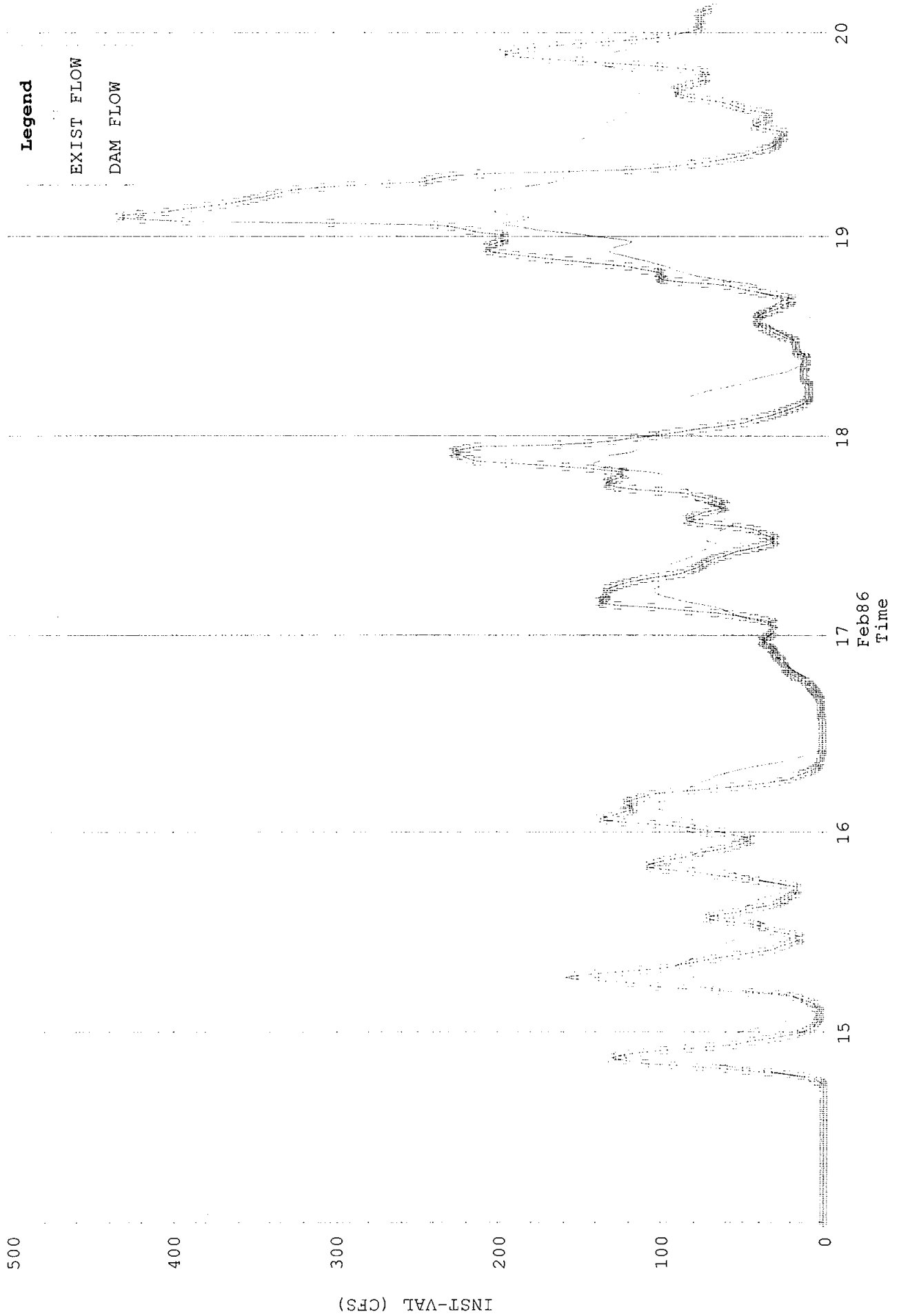
-----DSS---ZCLOSE Unit: 71, File: 2PIAIR.DSS

Pointer Utilization: 0.29
 Number of Records: 105
 File Size: 112.0 Kbytes
 Percent Inactive: 0.0

1986 DAM1, EXIST FLOW
Proposed Damsite Hydrographs; Evans Creek Block N; Airport Gage Data with AMC-III CN's



1986 SIERRA FLOW
Sierra Street Hydrographs; Evans Creek Block N; Airport Gage Data with AMC-III CN'S



1986-REGRESSION, 24 HOUR

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*DIAGRAM
1         ID    PHASE II SUB-AREAS
2         ID
3         ID    EVANS CREEK BLOCK N HYDROLOGY          NIMBUS JOB # 9912
4         ID    1986 EVENT-24 HOUR                   DATE : MAY 1999
5         ID    NIMBUS ENGINEERS, RENO, NEVADA        FILE NAME: 2regPH.DAT
6         ID
7         ID
8         ID    THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
9         ID    *****
10        ID    GONI CANYON                            Nimbus Job #: 9606      *
11        ID    Prepared by Nimbus Engineers           Date: August 1996   *
12        ID                                           File Name: GONI-5.DAT *
13        ID    *****
14        ID
15        ID    The PH card was developed from a regression of the hourly airport data.
16        ID
17        ID    *****
18        ID    THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
19        ID    EXISTING CONDITIONS (PLAN 2)                                           *
20        ID    *****
21        ID    CURVE NUMBERS WERE MODIFIED FROM AMC-II CONDITIONS TO AMC-III CONDITIONS
22        ID    IN ORDER TO MODEL A SATURATED WATERSHED. REFERENCE: MCCUEN, R.H. (1982);
23        ID    "A GUIDE TO HYDROLOGIC ANALYSIS USING SCS METHODS", PRENTICE-HALL, INC.
24        ID    *
24        IT    5 01MAY99    0005    288
25        IO    4    0
26        JP    2
27        *    DEPTH AREA REDUCTION FACTOR FROM NOAA
27        JR    PREC    0.99
28        *
28        KK    E1 SUBBASIN E1
29        BA    0.772
30        PH                0.01    0.04    0.15    0.25    0.39    0.92    2.33    3.67
31        LS                91
32        UD    0.79
33        *    ZW A=AIRPORT,24-HR C=FLOW F=E1FLOW
34        *
33        KK    E2 SUB-BASIN E2
34        BA    1.2
35        LS                92
36        UD    0.99
37        *    ZW A=AIRPORT,24-HR C=FLOW F=E2FLOW
38        *
37        KK    CP1 COMBINE E1 AND E2 HYDROGRAPHS AT CP1
38        *    KO                2
38        HC    2
39        *    ZW A=AIRPORT,24-HR C=FLOW F=E1+E2
40        *

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

39 KK RCH1 ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3

40 RD 10800 0.026 0.07 TRAP 20 2

*

41 KK E3 SUBBASIN E3

42 BA 1.51

43 LS 95

44 UD 1.24

* ZW A=AIRPORT,24-HR C=FLOW F=E3FLOW

*

45 KK CP2 COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3

46 HC 2

* ZW A=AIRPORT,24-HR C=FLOW F=E1+E2+E3

*

47 KK DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS

* KO 2

* KP 1

* KM PROPOSED RESERVOIR; flow through principal and emergency spillways ONLY.

* RS 1 STOR 0

* SA	.1354	.9137	1.954	3.63	3.71	3.79	3.87	3.95	5.12	6.
* SA	7.97	8.92	9.93	10.18	10.45	10.94	11.55	12.67	14.15	15.
* SE	765	775	785	795.89	796.33	796.76	797.2	797.64	803.16	808.
* SE	814.22	816.98	819.74	820.36	820.97	822.07	823.42	825.87	828.94	832
* SQ	0	0	0	0	13.46	38.09	69.97	107.73	114.35	120.
* SQ	126.56	129.43	132.24	218.16	401.01	881.31	1708.7	3654.6	6920.1	11031

* ZW A=AIRPORT,24-HR C=FLOW F=DAMFLOW

* ZW C=STOR F=DAMSTOR

* ZW C=STAGE F=DAMSTAGE

*

48 KP 1

49 KM PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

50 RS 1 STOR 0

51 SA	1.18	1.21	1.23	1.27	1.32	1.41	1.50	1.59	1.70	1.91
52 SA	2.83	3.91	3.97	4.72	6.68	9.07	10.37	11.09	12.75	15.70
53 SE	775.4	775.7	776.0	776.4	777.0	778.0	779.0	780.0	781.0	783.0
54 SE	790	796	796.3	800.4	808.69	816.98	820.36	822.10	825.87	832.0
55 SQ	5.4	12.6	21.4	35	40.8	49.9	57.5	64.3	70.4	81.3
56 SQ	92.1	95.43	112.2	134	145	155.4	244.5	907.5	3680.8	11060

57 ZW A=AIRPORT,24-HR C=FLOW F=30CMP

58 ZW C=STOR F=30CMP

59 ZW C=STAGE F=30CMP

*

* KO 2

60 KP 2

61 KM EXISTING CONDITIONS (NO RESERVOIR)

62 RN

63 ZW A=AIRPORT,24-HR B=EXIST C=FLOW F=EXISTFLOW

*

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
64	KK RCH3 ROUTE TO CP3
65	KM ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
66	KM HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES
67	RD 4200 0.023 0.09 TRAP 20 2
	*
68	KK E4 SUBBASIN E4
69	BA 0.58
70	LS 93
71	UD 0.57
	* ZW A=AIRPORT,24-HR C=FLOW F=E4FLOW
	*
72	KK CP3 COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
	* KO 2
73	HC 2
74	ZW A=AIRPORT,24-HR C=FLOW F=E4+CP2
	*
75	KK RCH3 ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
76	RD 3400 0.019 0.045 TRAP 20 2
	*
77	KK E5 SUB-BASIN E5
78	BA 0.31
79	LS 94
80	UD 0.5
	* ZW A=AIRPORT,24-HR C=FLOW F=E5FLOW
	*
81	KK CP4 COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH
	* KO 2
82	KM FLOW AT CP4 AT 43"X67" PAVED CMP AT SIERRA STREET
83	HC 2
	* ZW A=AIRPORT,24-HR C=FLOW F=SIERRA STREET
	*
84	KK SIERRA STORAGE AND DISCHARGE OF CULVERT AT SIERRA STREET
85	KM STORAGE ROUTING AT SIERRA STREET
86	RS 1 STOR 0
87	SA 0 0.51 0.76 1.2 1.88 2.59 3.13
88	SE 587 594 596 598 600 602 604
89	SQ 0 193 211 229 245 637 1000
90	ZW A=AIRPORT,24-HR C=FLOW F=SIERRA STREET
	*
91	ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

PUT
 INE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

28 E1
 .
 .
 33 . E2
 .
 .
 37 CP1.....
 V
 V
 39 RCH1
 .
 .
 41 . E3
 .
 .
 45 CP2.....
 V
 V
 47 DAM1
 V
 V
 64 RCH3
 .
 .
 68 . E4
 .
 .
 72 CP3.....
 V
 V
 75 RCH3
 .
 .
 77 . E5
 .
 .
 81 CP4.....
 V
 V
 84 SIERRA

(**) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                   *
*
* RUN DATE 06/30/1999 TIME 11:03:26 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS    *
* HYDROLOGIC ENGINEERING CENTER   *
*   609 SECOND STREET             *
*   DAVIS, CALIFORNIA 95616       *
*   (916) 756-1104                *
*
*****

```

PHASE II SUB-AREAS

```

EVANS CREEK BLOCK N HYDROLOGY      NIMBUS JOB # 9912
1986 EVENT-24 HOUR                 DATE : MAY 1999
NIMBUS ENGINEERS, RENO, NEVADA      FILE NAME: 2regPH.DAT

```

```

THIS HEC-1 MODEL IS MODIFIED FROM THE FOLLOWING HEC-1 MODEL PREPARED BY NIMB
*****
GONI CANYON                         Nimbus Job #: 9606      *
Prepared by Nimbus Engineers         Date: August 1996    *
                                     File Name: GONI-5.DAT *
*****

```

The PH card was developed from a regression of the hourly airport data.

```

*****
THIS MODEL IS FOR PROPOSED (PLAN 1) WITH 30" CMP RISER AND      *
EXISTING CONDITIONS (PLAN 2)                                     *
*****
CURVE NUMBERS WERE MODIFIED FROM AMC-II CONDITIONS TO AMC-III CONDITIONS
IN ORDER TO MODEL A SATURATED WATERSHED. REFERENCE: McCUEN, R.H. (1982);
"A GUIDE TO HYDROLOGIC ANALYSIS USING SCS METHODS", PRENTICE-HALL, INC.

```

```

25 IO      OUTPUT CONTROL VARIABLES
          IPRNT      4  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT         HYDROGRAPH TIME DATA
          NMIN      5  MINUTES IN COMPUTATION INTERVAL
          IDATE     1MAY99 STARTING DATE
          ITIME     0005 STARTING TIME
          NQ        288 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    2MAY99 ENDING DATE
          NDTIME    0000 ENDING TIME
          ICENT     19  CENTURY MARK

```

```

          COMPUTATION INTERVAL      .08 HOURS
          TOTAL TIME BASE           23.92 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

```

JP         MULTI-PLAN OPTION
          NPLAN      2  NUMBER OF PLANS

```

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 .99

 * *
 28 KK * E1 * SUBBASIN E1
 * *

SUBBASIN RUNOFF DATA

29 BA SUBBASIN CHARACTERISTICS
 TAREA .77 SUBBASIN AREA

PRECIPITATION DATA

30 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.01	.04	.15	.25	.39	.92	2.33	3.67	.00	.00	.00	.00

STORM AREA = .77

31 LS SCS LOSS RATE

STRTL	.20	INITIAL ABSTRACTION
CRVNBR	91.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

32 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.79	LAG
------	-----	-----

UNIT HYDROGRAPH
 49 END-OF-PERIOD ORDINATES

14.	45.	86.	140.	212.	297.	368.	417.	444.	448.
443.	416.	385.	348.	303.	250.	205.	174.	147.	125.
108.	92.	79.	65.	56.	48.	41.	34.	29.	24.
21.	18.	15.	13.	11.	9.	8.	7.	6.	5.
4.	4.	3.	3.	2.	2.	1.	1.	0.	

*** **

PLAN 2 INPUT DATA FOR STATION E1 ARE SAME AS FOR PLAN 1

 * *
 33 KK * E2 * SUB-BASIN E2
 * *

SUBBASIN RUNOFF DATA

34 BA SUBBASIN CHARACTERISTICS
TAREA 1.20 SUBBASIN AREA

PRECIPITATION DATA

30 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
..... HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
.01 .04 .15 .25 .39 .92 2.33 3.67 .00 .00 .00 .00

STORM AREA = 1.20

35 LS SCS LOSS RATE
STRTL .17 INITIAL ABSTRACTION
CRVNBR 92.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

36 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .99 LAG

UNIT HYDROGRAPH
61 END-OF-PERIOD ORDINATES

14.	41.	78.	122.	178.	250.	334.	413.	478.	525.
553.	560.	559.	546.	518.	486.	450.	408.	359.	306.
261.	228.	200.	175.	153.	137.	120.	106.	92.	80.
71.	62.	55.	48.	42.	37.	32.	28.	25.	22.
19.	17.	15.	13.	11.	10.	8.	8.	7.	6.
5.	5.	4.	4.	3.	3.	2.	2.	1.	1.
0.									

*** **

PLAN 2 INPUT DATA FOR STATION E2 ARE SAME AS FOR PLAN 1

* *
7 KK * CP1 * COMBINE E1 AND E2 HYDROGRAPHS AT CP1
* *

8 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

* *
39 KK * RCH1 * ROUTE E1+E2 HYDROGRAPH TO CP2 AT END OF SUB-BASIN E3
* *

HYDROGRAPH ROUTING DATA

40 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L	10800.	CHANNEL LENGTH
S	.0260	SLOPE
N	.070	CHANNEL ROUGHNESS COEFFICIENT
CA	.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	20.00	BOTTOM WIDTH OR DIAMETER
Z	2.00	SIDE SLOPE

*** **

PLAN 2 INPUT DATA FOR STATION RCH1 ARE SAME AS FOR PLAN 1

 * *
 41 KK * E3 * SUBBASIN E3
 * *

SUBBASIN RUNOFF DATA

42 BA SUBBASIN CHARACTERISTICS
 TAREA 1.51 SUBBASIN AREA

PRECIPITATION DATA

30 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

.....	HYDRO-35	TP-40	TP-49
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.01	.04	.15	.25	.39	.92	2.33	3.67	.00	.00	.00	.00

STORM AREA = 1.51

43 LS SCS LOSS RATE

STRTL	.11	INITIAL ABSTRACTION
CRVNBR	95.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

44 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	1.24	LAG
------	------	-----

UNIT HYDROGRAPH
 76 END-OF-PERIOD ORDINATES

11.	29.	55.	88.	125.	170.	227.	289.	360.	421.
476.	517.	545.	564.	568.	567.	562.	539.	515.	489.
460.	427.	390.	346.	304.	267.	240.	215.	193.	173.
156.	143.	129.	116.	105.	94.	83.	76.	68.	61.
55.	50.	44.	40.	36.	32.	29.	26.	23.	21.
19.	17.	15.	14.	12.	11.	10.	9.	8.	7.
7.	6.	6.	5.	5.	4.	4.	3.	3.	3.
2.	2.	1.	1.	1.	0.				

*** **

PLAN 2 INPUT DATA FOR STATION E3 ARE SAME AS FOR PLAN 1

* **

* *
45 KK * CP2 * COMBINE ROUTED E1+E2 HYDROGRAPH WITH HYDROGRAPH FROM E3
* *

46 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

* *
47 KK * DAM1 * PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
* *

* **

48 KP PLAN 1 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
PROPOSED RESERVOIR; flow through 30"CM Riser, principal and emergency spillwa

HYDROGRAPH ROUTING DATA

50 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP STOR TYPE OF INITIAL CONDITION
RSVRIC .00 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

51 SA	AREA	1.2	1.2	1.2	1.3	1.3	1.4	1.5	1.6	1.7	1.9
		2.8	3.9	4.0	4.7	6.7	9.1	10.4	11.1	12.8	15.7

53 SE	ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
		790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

35 SQ	DISCHARGE	5.	13.	21.	35.	41.	50.	58.	64.	70.	81.
		92.	95.	112.	134.	145.	155.	245.	908.	3681.	11060.

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	.36	.72	1.22	2.00	3.37	4.82	6.37	8.01	11.62
ELEVATION	775.40	775.70	776.00	776.40	777.00	778.00	779.00	780.00	781.00	783.00
STORAGE	28.10	48.24	49.42	67.21	114.23	179.26	212.09	230.76	275.66	362.70
ELEVATION	790.00	796.00	796.30	800.40	808.69	816.98	820.36	822.10	825.87	832.00

-----DSS---ZOPEN: Version: 6-EA; Existing File Opened
Unit: 71, File: 2REGPH.DSS
-----DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/DAM1/FLOW/01MAY1999/5MIN/30CMP/
-----DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/DAM1/STOR/01MAY1999/5MIN/30CMP/
-----DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/DAM1/STAGE/01MAY1999/5MIN/30CMP/

* *** **

60 KP PLAN 2 FOR STATION DAM1 PROPOSED RESERVOIR PER NRCS DESIGN AND EXISTING CONDITIONS
EXISTING CONDITIONS (NO RESERVOIR)

HYDROGRAPH ROUTING DATA

62 RN NO ROUTING

----DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/EXIST/FLOW/01MAY1999/5MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/EXIST/STOR/01MAY1999/5MIN/EXISTFLOW/
--DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/EXIST/STAGE/01MAY1999/5MIN/EXISTFLOW/

* *** **

64 KK * RCH3 * ROUTE TO CP3

ROUTE RESERVOIR/EXISTING DISCHARGE TO CP3
HEAVY RIPARIAN WITH WILLOWS, CATTAILS, WEEDS AND SEDGES

HYDROGRAPH ROUTING DATA

67 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L 4200. CHANNEL LENGTH
S .0230 SLOPE
N .090 CHANNEL ROUGHNESS COEFFICIENT
CA .00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 20.00 BOTTOM WIDTH OR DIAMETER
Z 2.00 SIDE SLOPE

* *** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

*** **

68 KK * E4 * SUBBASIN E4

SUBBASIN RUNOFF DATA

69 BA SUBBASIN CHARACTERISTICS

TAREA .58 SUBBASIN AREA

PRECIPITATION DATA

0 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.01	.04	.15	.25	.39	.92	2.33	3.67	.00	.00	.00	.00

STORM AREA = .58

70 LS SCS LOSS RATE
 STRTL .15 INITIAL ABSTRACTION
 CRVNBR 93.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

71 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .57 LAG

UNIT HYDROGRAPH
 36 END-OF-PERIOD ORDINATES

25.	76.	148.	254.	362.	431.	456.	454.	418.	371.
312.	240.	188.	149.	121.	98.	79.	63.	50.	40.
32.	25.	21.	17.	13.	11.	9.	7.	5.	5.
4.	3.	2.	2.	1.	0.				

* *** **

PLAN 2 INPUT DATA FOR STATION E4 ARE SAME AS FOR PLAN 1

** *** **

72 KK * CP3 * COMBINE ROUTED CP2 DISCHARGE WITH E4 HYDROGRAPH AT CP3
 * *

73 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

---DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/CP3/FLOW/01MAY1999/5MIN/E4+CP2/
 ---DSS---ZWRITE Unit 71; Vers. 3: /AIRPORT/CP3/FLOW/01MAY1999/5MIN/EXISTFLOW/

* *** **

75 KK * RCH3 * ROUTE CP3 HYDROGRAPH TO CP4 AT 43"X67" PAVED CMP AT RANCHO SAN RAFAEL/S
 * *

HYDROGRAPH ROUTING DATA

6 RD MUSKINGUM-CUNGE CHANNEL ROUTING
 L 3400. CHANNEL LENGTH
 S .0190 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA .00 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 20.00 BOTTOM WIDTH OR DIAMETER

*** **

PLAN 2 INPUT DATA FOR STATION RCH3 ARE SAME AS FOR PLAN 1

77 KK * E5 * SUB-BASIN E5

SUBBASIN RUNOFF DATA

78 BA SUBBASIN CHARACTERISTICS
TAREA .31 SUBBASIN AREA

PRECIPITATION DATA

30 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.01	.04	.15	.25	.39	.92	2.33	3.67	.00	.00	.00	.00

STORM AREA = .31

79 LS SCS LOSS RATE

STRTL	.13	INITIAL ABSTRACTION
CRVNER	94.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

80 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.50	LAG
------	-----	-----

UNIT HYDROGRAPH
32 END-OF-PERIOD ORDINATES

19.	55.	113.	189.	248.	274.	274.	251.	219.	175.
129.	100.	77.	62.	48.	37.	29.	23.	18.	14.
11.	8.	6.	5.	4.	3.	3.	2.	2.	1.
1.	0.								

** **

PLAN 2 INPUT DATA FOR STATION E5 ARE SAME AS FOR PLAN 1

81 KK * CP4 * COMBINE ROUTED DISCHARGE FROM CP3 WITH E5 HYDROGRAPH

ROUTED TO	RCH3	4.06	1	FLOW	485.
				TIME	19.33
			2	FLOW	577.
				TIME	18.33

HYDROGRAPH AT	E5	.31	1	FLOW	48.
				TIME	18.00
			2	FLOW	48.
				TIME	18.00

2 COMBINED AT	CP4	4.37	1	FLOW	509.
				TIME	19.33
			2	FLOW	621.
				TIME	18.25

ROUTED TO	SIERRA	4.37	1	FLOW	504.
				TIME	19.58
			2	FLOW	619.
				TIME	18.33

** PEAK STAGES IN FEET **

1	STAGE	601.32
	TIME	19.58
2	STAGE	601.91
	TIME	18.33

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR PLAN = 1 RATIO= .00

RCH1	MANE	5.00	283.07	1115.00	2.53	5.00	283.07	1115.00	2.53
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2737E+03 EXCESS= .0000E+00 OUTFLOW= .2660E+03 BASIN STORAGE= .8342E+01 PERCENT ERROR= -.2

FOR PLAN = 2 RATIO= .00

RCH1	MANE	5.00	283.07	1115.00	2.53	5.00	283.07	1115.00	2.53
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2737E+03 EXCESS= .0000E+00 OUTFLOW= .2660E+03 BASIN STORAGE= .8342E+01 PERCENT ERROR= -.2

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	439.31	1160.00	1.52	5.00	439.31	1160.00	1.52
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2894E+03 EXCESS= .0000E+00 OUTFLOW= .2839E+03 BASIN STORAGE= .5659E+01 PERCENT ERROR= -.1

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	501.94	1115.00	2.65	5.00	501.94	1115.00	2.65
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4993E+03 EXCESS= .0000E+00 OUTFLOW= .4938E+03 BASIN STORAGE= .5919E+01 PERCENT ERROR= -.1

FOR PLAN = 1 RATIO= .00

RCH3	MANE	5.00	484.98	1160.00	1.69	5.00	484.98	1160.00	1.69
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3700E+03 EXCESS= .0000E+00 OUTFLOW= .3666E+03 BASIN STORAGE= .3415E+01 PERCENT ERROR= .0

FOR PLAN = 2 RATIO= .00

RCH3	MANE	5.00	577.34	1100.00	2.66	5.00	577.34	1100.00	2.66
------	------	------	--------	---------	------	------	--------	---------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5799E+03 EXCESS= .0000E+00 OUTFLOW= .5768E+03 BASIN STORAGE= .3569E+01 PERCENT ERROR= -.1

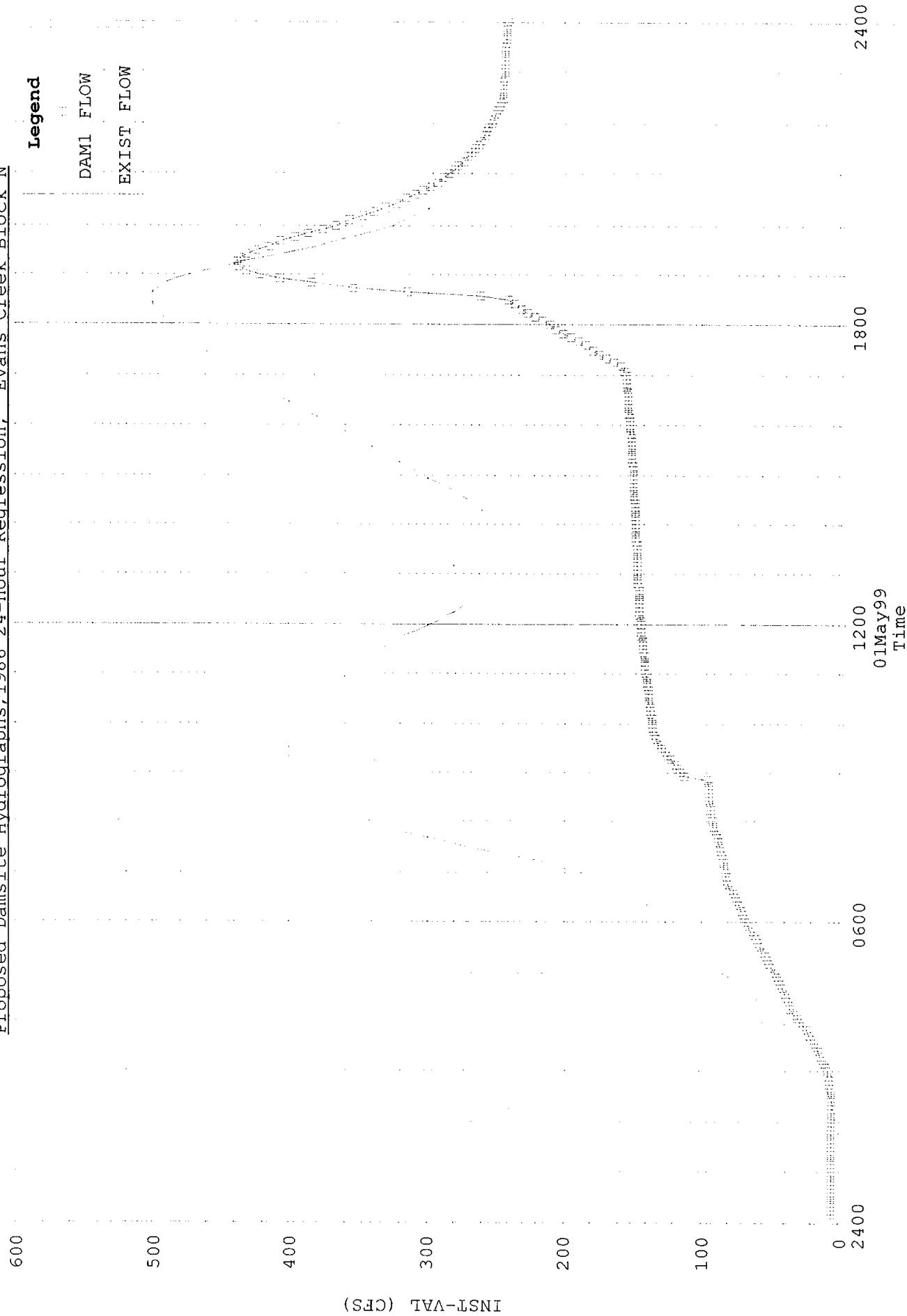
NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 2REGPH.DSS

Pointer Utilization: .25
Number of Records: 10
File Size: 25.1 Kbytes
Percent Inactive: .00

AIRPORT DAM1, EXIST FLOW

Proposed Damsite Hydrographs; 1986 24-hour Regression; Evans Creek Block N



AIRPORT SIERRA FLOW

Sierra Street Hydrographs;1986 24-hour Regression; Evans Creek Block N



AIRPORT DAM1 STAGE

Proposed Damsite Stage; 1986 24-hour Regression; Evans Creek Block N

